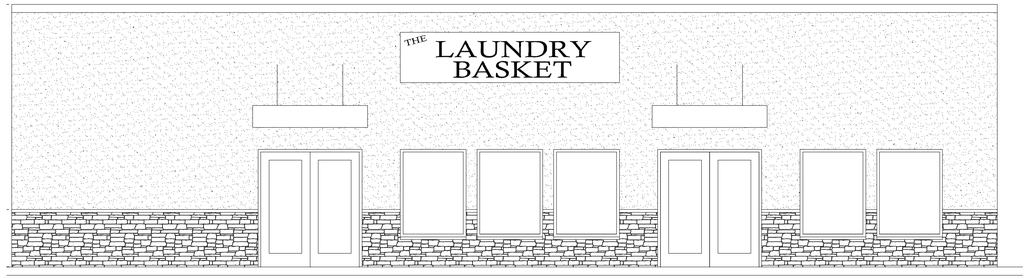
WILLMAR LAUNDRY

WILLMAR, MN



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CIVIL

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C4.0 GRADING & EROSION CONTROL PLAN
C5.0 CONSTRUCTION DETAILS

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A102 REFLECTED CEILING PLAN
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MECHANICAL

DESIGN BUILD BY CONTRACTOR

ELECTRICAL

DESIGN BUILD BY CONTRACTOR

Project Team

OWNER

WILLMAR LAUNDRY
RYAN AND MARIE VATNSDAL

CIVIL ENGINEER

Hagstrom Enginering
219 6th Avenue East
Alexandria, MN
Contact: Matt Hagstrom

ARCHITECT

Ringdahl Architects, INC.
510 22nd Ave East Suite #102
Alexandria, MN 56308
Contact: ***Dustin Tomoson***
Email: dustin@ringdahlarchitects.com
Phone: 320-763-9368

STRUCUTRAL ENGINEER

Design Tree Engineering
120 17th Avenue West
Alexandria, MN 56308

Mech/Elec/Plumb.

MBN ENGINEERING
701 Main Avenue
Fargo, ND 58103

CODE SUMMARY

1. Building Description:
   a. 3,600 sf footprint, slab on grade wood framed building.
2. Code Summary
   a. 2020 Minnesota Building Code
   b. 2020 Minnesota Accessibility Code
   c. 2024 Minnesota Energy Code
3. Occupancy (302)
   A. (B), Laundromat (3,600 sf)
4. Occupancy Separation:
   a. Single occupancy - no seperation required.
5. Type of Construction:
   a. V-B (Combustible)
6. Location on Property:
   a. North: 42'-6"
   b. South: 46'-7"
   c. East: 99'-11"
   d. West 90'-0"
7. Allowable Floor Area/Height:
   a. 9,000 square feet
8. Exterior wall protection: None required > 10' (Table 602).
9. Exterior opening protection: None required if exterior wall rating not required (Table 705.8), not required.
10. Fire Blocking (718): 10' intervals in concealed spaces of combustible construction, horizontally and vertically
11. Sprinkler Fire Protection:
    a. Less than 9,000 square foot B occupancy - no sprinkler system requried.
12. Exiting, 1006:

    First Floor: (24 occupants)
    a. B, (Office): 3,600 sf/150 sf/occupant = 24 occupants
       1 exit required (Table 1006.2.1) < 49 occupants
       200' maximum distance to exit (Table 1017.2 w/o sprinkler)
13. Accessibility: Required (Chapter 11)
    a. Chapter 11 requires accessibiltiy at all spaces except mechanical room.
14. Minimum Plumbing Fixtures: (Table 2902.1)
    Business: One fixture per 25 occupants for first 50 occupants, one per 50 thereafter: One lavatory per 40 occupants for first 80 occupants, one per 80 thereafter, (fixtures distributed equally between sexes)

	Criteria	Required
(B Occupancy 24 occ)		
Water Closets (1/25 occ)	1/25	0.96
Lavatories (1/40 occ)	1/40	0.6

    Water Closets Require: 0.96 = 1 water closet required
    Lavatories Required: 0.60 - 1 lavatory required

    OTHER FIXTURES
    Drinking Fountain: Not requires, less than 50 occupants.
    Service Sink 1 required. Provide scrubbable surface at adjacent walls.
15. Illuminated Exit Signs: Required in areas which require more than 1 exit: not required (1013).
16. Egress Illumination: Required in areas which require more than 1 exit: not required (1008).
17. Energy Code Analysis: (Zone 6 Commercial)
    A) Roof: Wood Framed Building
       i. U-0.021
    B) Walls: Wood Building
       i. U-0.051
    C) Foundation:
       i. U-0.027
    D) Opaque doors: U0.37
    E) Overhead doors: R-4.75
    F) Windows:
       i. Fixed fenestrations: U0.34
       ii. Operable Fenestrations: U0.42
       iii. Entrance doors: U0.63
18. Zoning: City of Willmar

SITE MAP



RINGDAHL Architects, inc

Easton Place
510 22nd Ave. East, Suite 102
Alexandria, MN 56308
ph. (320) 763-9368
fax (320) 762-9063

WILLMAR
LAUNDRY
WILLMAR, MN

CODE REVIEW/
CODE PLAN

ISSUE FOR:	ISSUE DATE:
PERMIT	10/6/2026

I CERTIFY THAT THIS PLAN, SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED ARCHITECT UNDER THE LAWS OF THE STATE OF MINNESOTA.

10/6/2026 — DATE
REG. NO.

COMMISSION NO:	2635
DRAWN BY:	DLT
CHECKED BY:	PDR
DATE:	10/6/2026

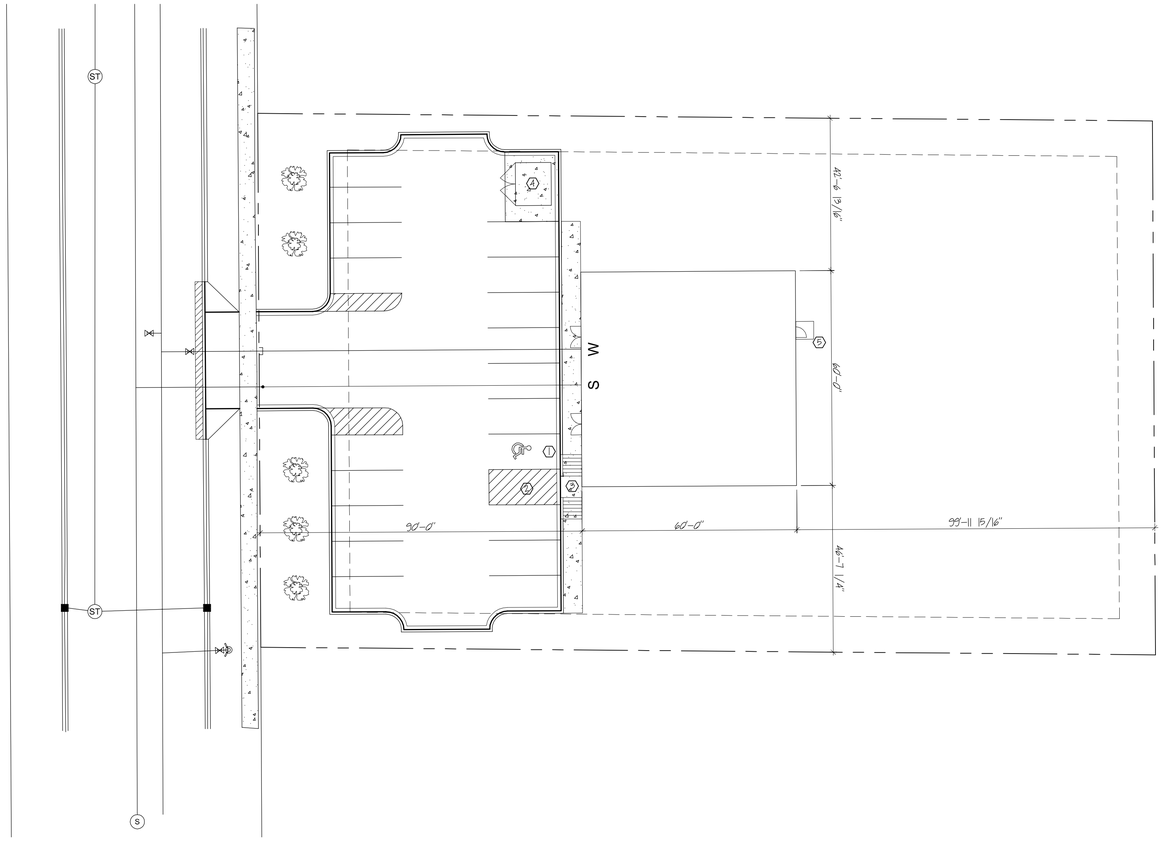
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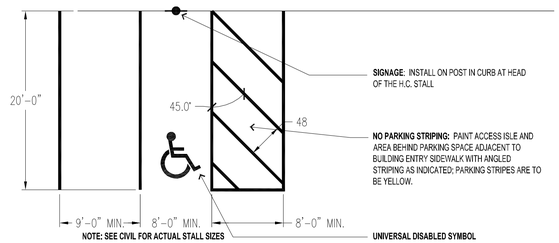
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KEY NOTES

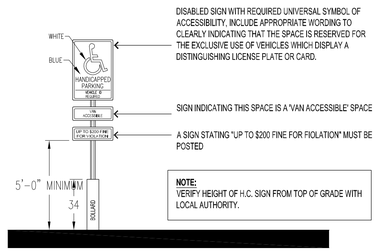
1. HANDICAPPED PARKING SIGN AT HEAD OF STALL
2. ACCESS AISLE WITH "NO PARKING" PAINTED ON STALL
3. ADA CURB CUT - SEE CIVIL
4. 12'-0" x12'-0" TRASH ENCLOSURE - CHAIN LINK WITH VINYL SLATS
5. CONCRETE STOOP - SEE STRUCTURAL



1 C0.1 ARCHITECTURAL SITE PLAN
SCALE: 1" = 20'-0"



3 C0.1 H.C. STALL DETAIL
SCALE: 1/8" = 1'-0"



2 C0.1 H.C. STALL SIGNAGE
SCALE: 1/4" = 1'-0"

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Easton Place
510 22nd Ave. East, Suite 102
Alexandria, MN 56308
ph. (320) 763-9368
fax (320) 762-9063

WILLMAR LAUNDRY
WILLMAR, MN

ARCHITECTURAL SITE PLAN

ISSUE FOR:	ISSUE DATE:
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10/6/2026 DATE — 58291 REG. NO.

COMMISSION NO:	2635
DRAWN BY:	DLT
CHECKED BY:	PDR
DATE:	10/6/2026

DRAWING NO:
C0.1

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WILMAR LAUNDRY FACILITY
KANDIYOHI COUNTY, MN

CIVIL ENGINEERING CONSTRUCTION DOCUMENTS

SECTION 23, TOWNSHIP 119 N, RANGE 35 W
KANDIYOHI COUNTY, STATE OF MINNESOTA

CONTACT LIST

OWNER
MARIE & RYAN VATNSDAHL

CIVIL ENGINEER
HAGSTROM ENGINEERING
219 6TH AVENUE E
ALEXANDRIA, MN 56308
320-759-0764
CONTACT: MATT HAGSTROM



VICINITY MAP
N.T.S.

SHEET INDEX

C1.0 COVER SHEET
C3.0 SITE & UTILITY PLAN
C4.0 GRADING & EROSION CONTROL PLAN
C5.0 CONSTRUCTION DETAILS

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DATE: %% LICENSE NUMBER: 50731

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CIVIL ENGINEERS | LAND CONSULTANTS
219 6TH AVENUE EAST
ALEXANDRIA, MN 56308
P.320.759.0764
WWW.HAGSTROMENG.COM
FOR AND ON BEHALF OF HAGSTROM ENGINEERING PLLC

BENCHMARK:

CALL GOPHER STATE ONE CALL
811
CALL 2 BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

No.	Description of Revisions	Designed By: BCH	Checked By: MRH
		Date	By

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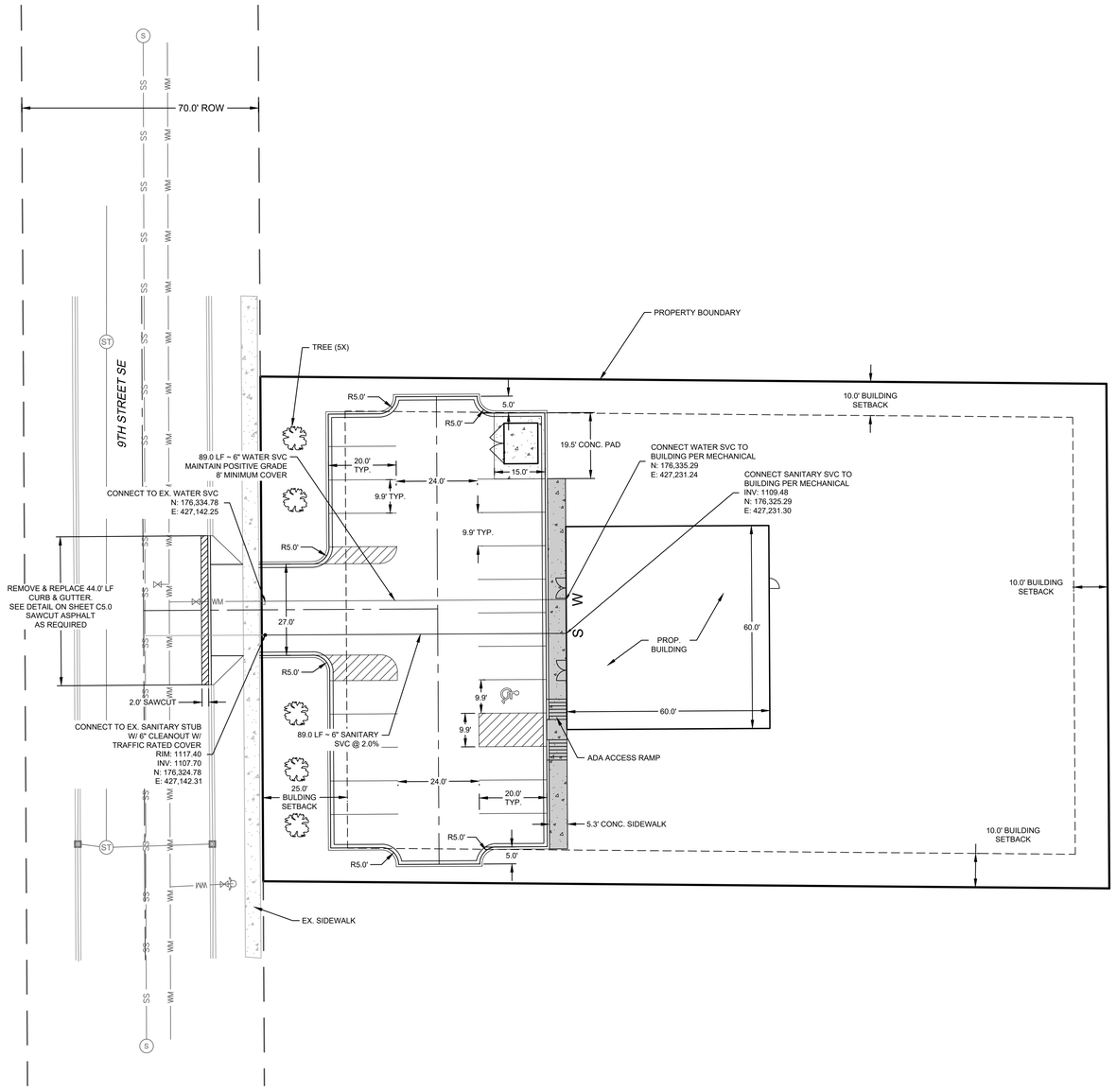
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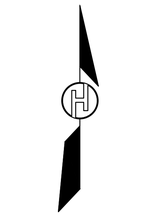
Date:
Job No.: 26027

C1.0

Sheet C1.0 of C5.0

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SCALE: 1" = 20'
ORIGINAL GRAPHIC SCALE

LEGEND

- EXISTING ROW
- PROPERTY BOUNDARY
- EXISTING SANITARY SEWER W/ MANHOLE
- EXISTING STORM SEWER W/ MANHOLE
- EXISTING WATER LINE W/ GATE VALVE
- PROPERTY SETBACK
- PROPOSED WATER SERVICE
- PROPOSED SANITARY SERVICE W/ CO
- PROPOSED CONCRETE
- PROPOSED CONCRETE SIDEWALK
- PROPOSED ASPHALT
- PROPOSED CURB & GUTTER

NOTES:

1. CONTRACTOR SHALL FIELD VERIFY SIZE AND LOCATION OF ALL EXISTING UTILITIES PRIOR TO CONSTRUCTION AND NOTIFY THE ENGINEER WITH ANY DISCREPANCIES.
2. CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL NECESSARY CONSTRUCTION STAKING.
3. INSTALL PERIMETER BMPs PRIOR TO ANY LAND DISTURBING ACTIVITIES, SEE EROSION CONTROL PLAN.
4. ALL CURB AND GUTTER IS B612 CATCH CURB.
5. ALL SANITARY LINES SHALL MEET ASTM D3034 QUALITY STANDARDS.
6. ALL WATER LINES SHALL MEET ASTM D1785 QUALITY STANDARDS AND MAINTAIN A POSITIVE SLOPE.
7. MAINTAIN MINIMUM 8' COVER OVER SANITARY SEWER AND WATER LINES.
8. INSTALL ALL PIPE WITH LESS THAN 8' COVER.
9. SEE SHEET C5.0 FOR CONSTRUCTION DETAILS.

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Checked By: MRH

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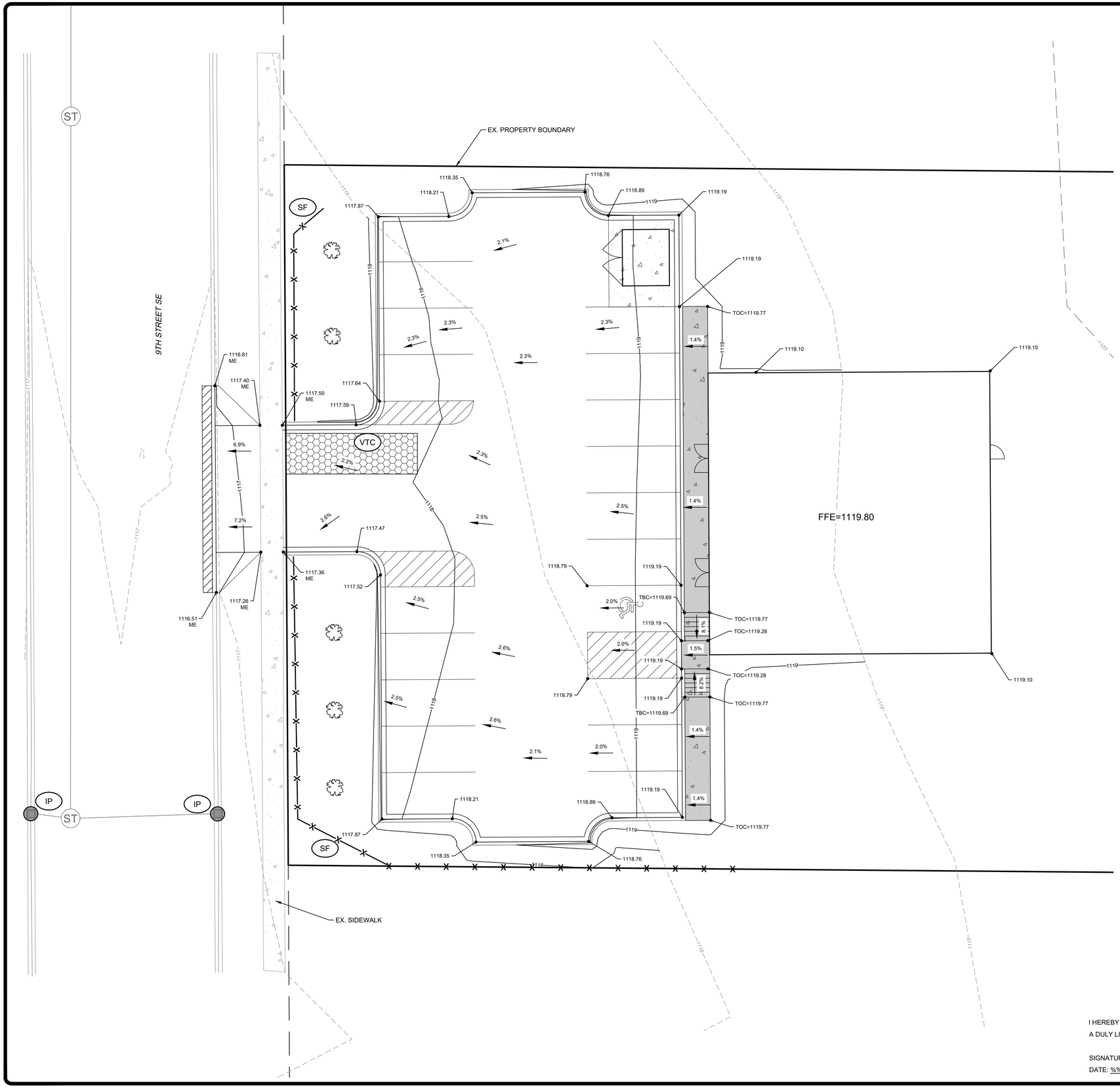
SITE & UTILITY PLAN

Date:
Job No.: 26027

C3.0

Sheet C3.0 of C5.0

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0 5 10 20

SCALE: 1" = 10'
ORIGINAL GRAPHIC SCALE

LEGEND

1431 / 1430	EXISTING 1' AND 5' CONTOURS
1431 / 1430	PROPOSED 1' AND 5' CONTOURS
1432.3	PROPOSED SPOT ELEVATION
2.00%	FLOW DIRECTION & GRADE
FFE	FINISHED FLOOR ELEVATION
ME	MATCH EXISTING
TOC	TOP OF CONCRETE
TBC	TOP BACK OF CURB
	LIMITS OF GRADING
VTC	VEHICLE TRACKING CONTROL, 1 EA
SF	SILT FENCE, 200 LF
IP	INLET PROTECTION, 2 EA

GRADING NOTES:

1. CONTRACTOR SHALL FIELD VERIFY SIZE AND LOCATION OF ALL EXISTING UTILITIES PRIOR TO CONSTRUCTION AND NOTIFY THE ENGINEER WITH ANY DISCREPANCIES.
2. CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL NECESSARY CONSTRUCTION STAKING.
3. ALL ELEVATIONS PROVIDED ARE FINISHED GRADE UNLESS OTHERWISE NOTED.
4. FINISHED GRADE MUST SLOPE AWAY FROM BUILDING AT 10% MIN. FOR THE FIRST 5' IN LANDSCAPED AREAS.

EROSION CONTROL NOTES:

1. INSTALL PERIMETER BMPs PRIOR TO ANY CONSTRUCTION.
2. SEE SHEET C5.0 FOR BMP DETAILS AND BMP MAINTENANCE NOTES.
3. THE CONTRACTOR SHALL PERFORM STREET SWEEPING AS REQUIRED TO MITIGATE ANY SOIL ON THE PARKING LOT OR PERIMETER STREETS.
4. CONCRETE WASHOUT TO OCCUR OFFSITE.
5. THE CONTRACTOR SHALL INSPECT THE BMPs WEEKLY OR AFTER ANY SIGNIFICANT RAIN EVENT AND REPAIR BMPs AS REQUIRED.
6. STOCKPILE AND STAGING LOCATION TO BE APPROVED BY ENGINEER.
7. ALL DISTURBED AREAS THAT ARE NOT HARDSCAPED SHALL RECEIVE PERMANENT SEEDING PER OWNER.

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Checked By: MRH

WILMAR LAUNDRY FACILITY

GRADING & EROSION CONTROL PLAN

Date:
Job No.: 26027

C4.0

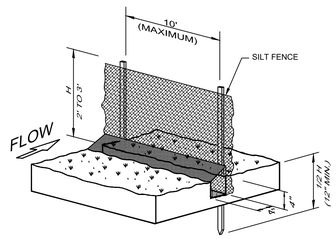
Sheet C4.0 of C5.0

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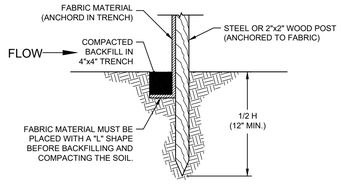
SILT FENCE
N.T.S. — SF

DEFINITION
A TEMPORARY SEDIMENT CONTROL CONSISTING OF A FILTER FABRIC STRETCHED ACROSS AND ATTACHED TO SUPPORTING POSTS AND ENTRENCHED.

PURPOSE
TO MINIMIZE SEDIMENT LADEN SHEET FLOW FROM LEAVING DISTURBED AREAS.



SILT FENCE INSTALLATION



SILT FENCE TRENCH SECTION

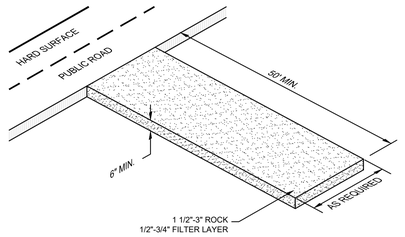
NOTES
1. SILT FENCING MUST NOT BE INSTALLED WHERE CONCENTRATED FLOWS WILL OCCUR (SUCH AS DRAINAGE CHANNELS, AROUND AREA DRAINS, OR UPSTREAM/DOWNSTREAM OF CULVERTS) UNLESS PROPERLY SUPPORTED.
2. MAXIMUM DRAINAGE AREA SHALL NOT EXCEED 0.25 ACRES PER 100-FT FENCE LENGTH. MAXIMUM SLOPE LENGTH ABOVE THE FENCE SHALL NOT EXCEED 100 FT.

VEHICLE TRACKING CONTROL
VTC

DEFINITION
A STONE STABILIZED PAD LOCATED AT POINTS OF VEHICULAR INGRESS AND EGRESS ON CONSTRUCTION SITE.

PURPOSES
TO REDUCE THE AMOUNT OF MUD TRANSPORTED ONTO PUBLIC ROADS BY MOTOR VEHICLES OR RUNOFF.

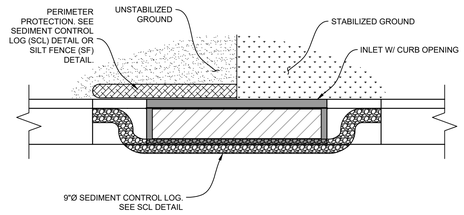
NOTE: ONLY APPLICABLE FOR SITES GREATER THAN 2 ACRES IN SIZE.



INLET PROTECTION - CURB
N.T.S. — IP

DEFINITION
A TEMPORARY SEDIMENT CONTROL MEASURE PLACED AROUND A STORM DRAIN CURB INLET.

PURPOSE
TO REDUCE THE AMOUNT OF SEDIMENT AND DEBRIS ENTERING STORM DRAINAGE SYSTEMS PRIOR TO PERMANENT STABILIZATION OF DISTURBED AREAS.

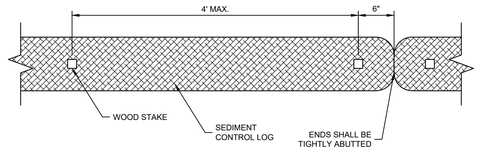


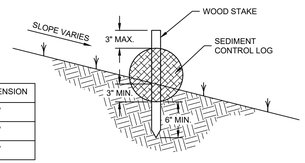
PLAN
N.T.S.

SEDIMENT CONTROL LOG
N.T.S. — SCL

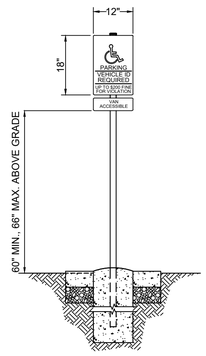
DEFINITION
A WATTLE WITH AN EXCELSIOR, STRAW, COMPOST, OR COCONUT FILL MATERIAL CONSTRUCTED TO CREATE A PERVIOUS FILTER, ABLE TO FACILITATE OVERTOPPING.

PURPOSE
TO PROMOTE SEDIMENTATION ON THE UPGRADIENT SIDE AND REDUCE STORM RUNOFF VELOCITY.

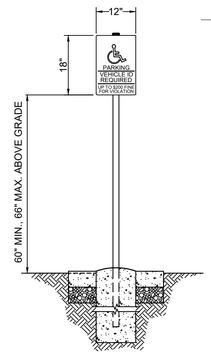




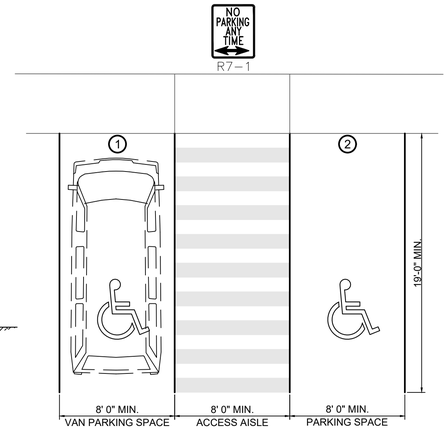
LOGØ	STAKE DIMENSION
9"Ø	1.5"x1.5"x18"
12"Ø	1.5"x1.5"x24"
20"Ø	1.5"x1.5"x48"



1 ADA SIGN
N.T.S.



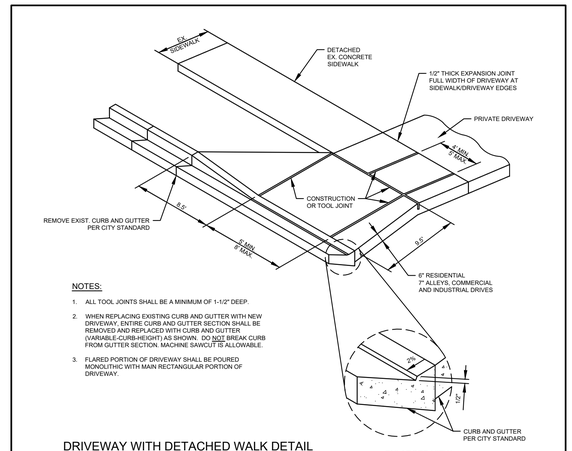
2 ADA SIGN
N.T.S.



ADA PARKING SPACE LAYOUT
N.T.S.

ICC 502.2
VEHICLE SPACE SIZE

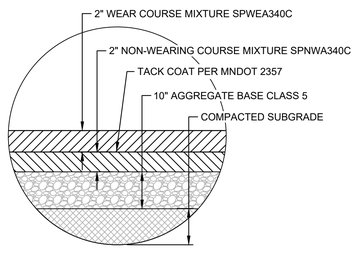
ICC 502.4.2
ACCESS AISLE WIDTH



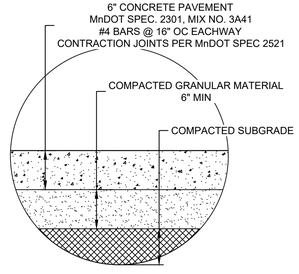
NOTES:
1. ALL TOOL JOINTS SHALL BE A MINIMUM OF 1-1/2" DEEP.
2. WHEN REPLACING EXISTING CURB AND GUTTER WITH NEW DRIVEWAY, ENTIRE CURB AND GUTTER SECTION SHALL BE REMOVED AND REPLACED WITH CURB AND GUTTER (VARIABLE-CURB-HEIGHT) AS SHOWN. DO NOT BREAK CURB FROM GUTTER SECTION. MACHINE SAWCUT IS ALLOWABLE.
3. FLARED PORTION OF DRIVEWAY SHALL BE POURED MONOLITHIC WITH MAIN RECTANGULAR PORTION OF DRIVEWAY.

DRIVEWAY WITH DETACHED WALK DETAIL
N.T.S.

ENLARGED VIEW



BITUMINOUS PAVEMENT SECTION
N.T.S.



CONCRETE PAD SECTION
N.T.S.

REINFORCING STEEL SHALL HAVE MIN. 3" BOTTOM COVER

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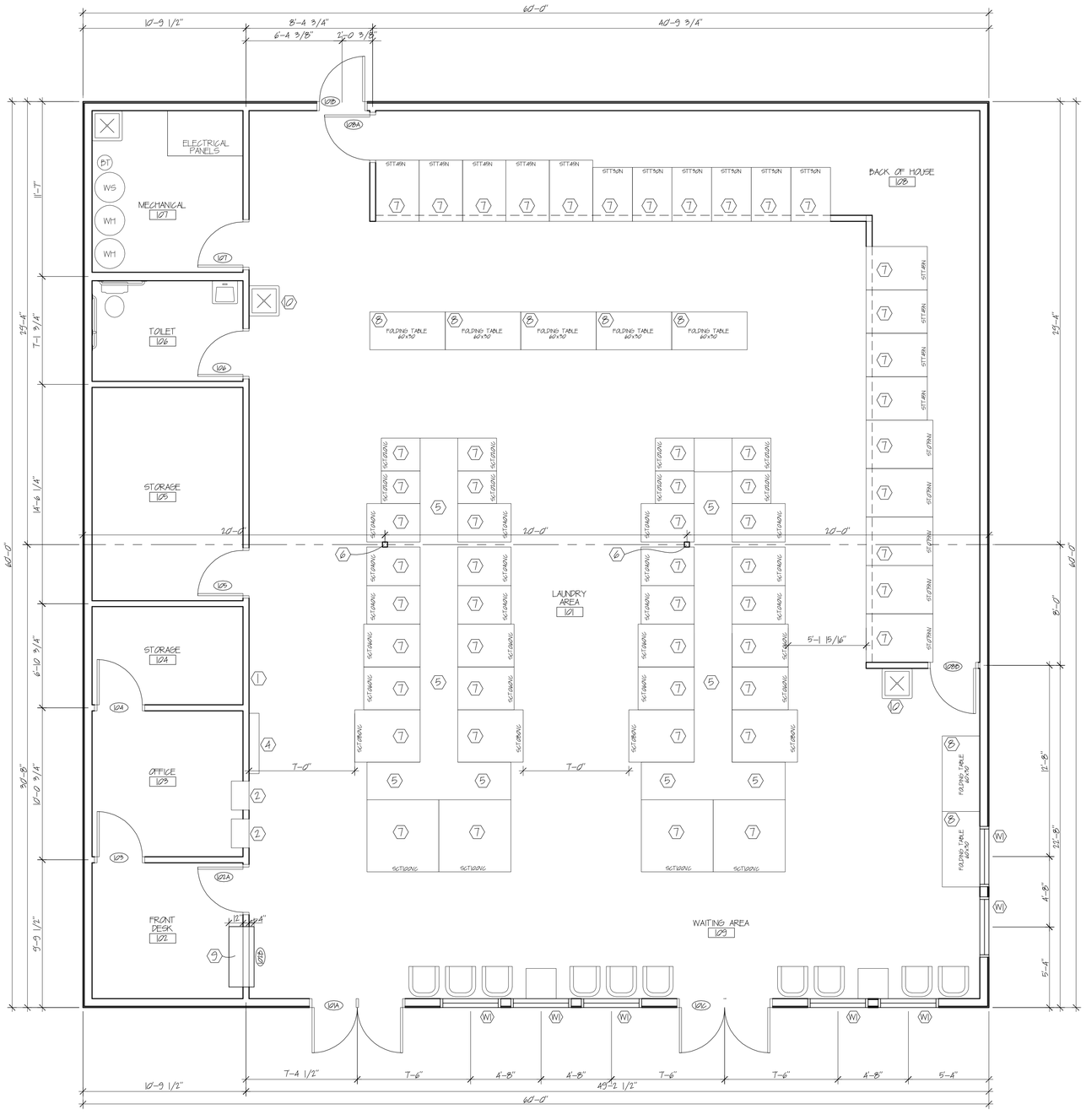
WILMAR
LAUNDRY FACILITY

CONSTRUCTION DETAILS

Date:
Job No.: 26027

C5.0

Sheet C5.0 of C5.0



1 A101 FLOOR PLAN
SCALE: 1/4" = 1'-0"

PLAN NOTES

1. VERIFY CONDITIONS & DIMENSIONS AND REPORT DISCREPANCIES TO ARCHITECT FOR RESOLUTION PRIOR TO COMMENCING WORK.
2. REFER TO STRUCTURAL, MECHANICAL & ELECTRICAL FOR ADDITIONAL NOTES & INFORMATION.
3. SEAL ALL MECHANICAL, ELECTRICAL, PENETRATIONS OF WALL/CEILING/FLOOR CONSTRUCTION.
4. DIMENSIONS TO FACE OF STUD, FACE OF CONCRETE BLOCK OR CENTERLINE OF STUD WALL.
5. PROVIDE 6" STUD WALLS @ ELECTRICAL PANELS AND PLUMBING WALLS (WHERE REQUIRED).
6. DO NOT SCALE DRAWINGS.

WALL FRAMING KEY

TYPICAL INTERIOR WALL	5/8" GYPSUM, 2x STUDS 16" OC., 5/8" GYPSUM
TYPICAL EXTERIOR WALL	5/8" GYPSUM, 6 MIL POLY V.B. 2x6 STUDS 16" OC., R-21 BATT INSUL., R-3 ZIP SHTG., EIFS/ STONE VENEER

DOOR AND WINDOW SCHEDULE

NO.	SIZE	DOOR TYPE	FRAME TYPE	HDWR.	REMARKS
101A	6'-0" x 7'-0"	AL	AL	36	CYLINDER (REMOVABLE MULLION)
101B	5'-0" x 7'-0"	HM	HM	32	CYLINDER
101C	5'-0" x 7'-0"	AL	AL	36	CYLINDER (REMOVABLE MULLION)
102A	5'-0" x 7'-0"	WD	WD	32	KEYPAD LOCK
103A	4'-0" x 4'-0"	AL	AL	-	COILING ALUMINUM DOOR
103	3'-0" x 7'-0"	HM	WD	32	KEYED LOCK/DEADBOLT
104	3'-0" x 7'-0"	WD	WD	32	PASSAGE
105	3'-0" x 7'-0"	WD	WD	32	KEYED LOCK
106	3'-0" x 7'-0"	WD	WD	32	PRIVACY LOCK
107	3'-0" x 7'-0"	WD	WD	32	KEYED LOCK
108A	3'-0" x 7'-0"	WD	WD	32	KEYED LOCK
108B	3'-0" x 7'-0"	WD	WD	32	KEYED LOCK
W1	4'-0" x 8'-0"	AL			

DOOR TYPES:
WD = WOOD FLUSH DOOR
AL = ALUMINUM

FRAME TYPES:
HM = HOLLOW METAL
AL = ALUMINUM
WD = WOOD

HARDWARE GROUPS:

GROUP 36 (ALUM)
A. CONT GEARED HINGE
B. PANIC BAR
C. CLOSER LCN 4010-4010
D. THRESHOLD 1/2"
E. SWEEP
F. WEATHERSTRIP

GROUP 32
A. BUTTS AS REQUIRED
B. 1 LOCKSET: SCHLAGE ALX40
C. STOP: OVERHEAD
D. CLOSER AS REQUIRED

KEY NOTES

1. FIRE EXTINGUISHER - 75' TRAVEL DISTANCE MAX.
2. COIN MACHINE - COORDINATE WITH OWNER
3. MOP SINK - PROVIDE RFP TO 48" MIN AT ADJACENT WALLS
4. LAUNDRY SUPPLIES DISPENSER - COORDINATE WITH OWNER
5. TRENCH FOR LAUNDRY EQUIPMENT - COORDINATE WITH OWNER
6. STRUCTURAL COLUMN - SEE STRUCTURAL (PAINT)
7. LAUNDRY EQUIPMENT BY OWNER - COORDINATE MECHANICAL AS REQUIRED.
8. FOLDING TABLE AT 34" A.F.F. BY OWNER
9. PLAM COUNTERTOP AT 34" A.F.F.
10. LAUNDRY TUB WITH RFP TO 48" A.F.F. AND 12" ADJACENT TO THE LAUNDRY TUB

NOTE: PROVIDE CHAIR RAIL AT LAUNDRY AREA AND WAITING AREAS.

Finish Materials Legend

Floors
CONC SEALED CONCRETE
LVP LUXARY VINYL PLANK

Base
VB 4" VINYL BASE

Walls
PT PAINT - COLOR BY OWNER
FRP CRANE COMPOSITES SEQUENTIA FLAT - 836 SMOOTH GRAY
PW PLYWOOD TO 8'-0" (PAINT)
FT GYPSUM FIRE TAPE ONLY

Ceilings
ACT USG 411 FROST SLD "WHITE" (USG DX/DXL BLACK GRID)
P PAINTED STRUCTURE

Room Finish Schedule

NO.	NAME	FLOOR FINISH		BASE FINISH	WALLS FINISH				CEILINGS FINISH		GENERAL REMARKS
	● ALL WALLS	CONC	LVP	VB	PT	FRP	PW	FT	ACT-1	P	
101	LAUNDRY AREA		●	●	●					●	
102	FRONT DESK		●	●	●				●		
103	OFFICE		●	●	●				●		
104	STORAGE	●		●				●		●	FIRE TAPE GYPSUM (NO PAINT)
105	STORAGE	●		●				●		●	FIRE TAPE GYPSUM (NO PAINT)
106	TOILET		●	●	●	●			●		FRP TO 8'-0" AT WET WALLS ONLY
107	MECHANICAL	●		●			●	●		●	FRP TO 48" AT MOP SINK
108	BACK OF HOUSE	●		●	●					●	FIRE TAPE GYPSUM (NO PAINT)
109	WAITING AREA		●	●	●					●	

RINGDAHL Architects, inc
Easton Place
510 22nd Ave. East, Suite 102
Alexandria, MN 56308
ph. (320) 763-9368
fax (320) 762-9063

WILLMAR LAUNDRY
WILLMAR, MN

FLOOR PLAN

ISSUE FOR:	ISSUE DATE:
PERMIT	10/6/2026

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10/6/2026 DATE — 18821 REG. NO.

COMMISSION NO:	2635
DRAWN BY:	DLT
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DATE:	10/6/2026

DRAWING NO.
A101

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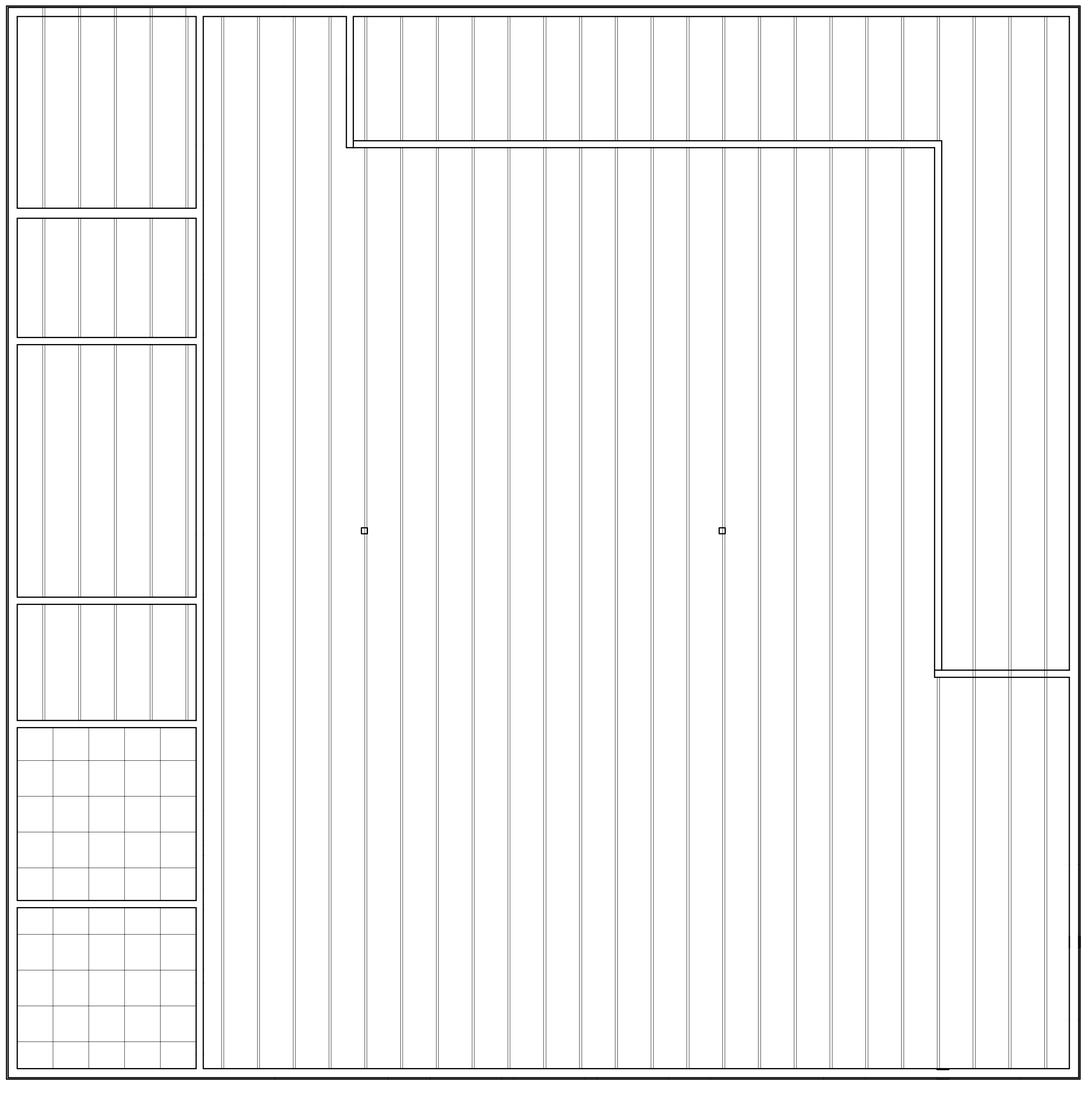
PLAN NOTES

1. VERIFY CONDITIONS & DIMENSIONS AND REPORT DISCREPANCIES TO ARCHITECT FOR RESOLUTION PRIOR TO COMMENCING WORK.
2. REFER TO MECHANICAL AND ELECTRICAL FOR ADDITIONAL NOTES & INFORMATION.
3. SEAL ALL MECHANICAL AND ELECTRICAL PENETRATIONS OF WALL/CEILING/FLOOR CONSTRUCTION.

KEY

ACOUSTIC CEILING TILE @ EL. 9'-0" (±) U.N.O.

OPEN TO STRUCTURE PAINT



1 A102 REFLECTED CEILING PLAN
SCALE: 1/4" = 1'-0"

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Easton Place
510 22nd Ave. East, Suite 102
Alexandria, MN 56308
ph. (320) 763-9368
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REFLECTED CEILING PLAN

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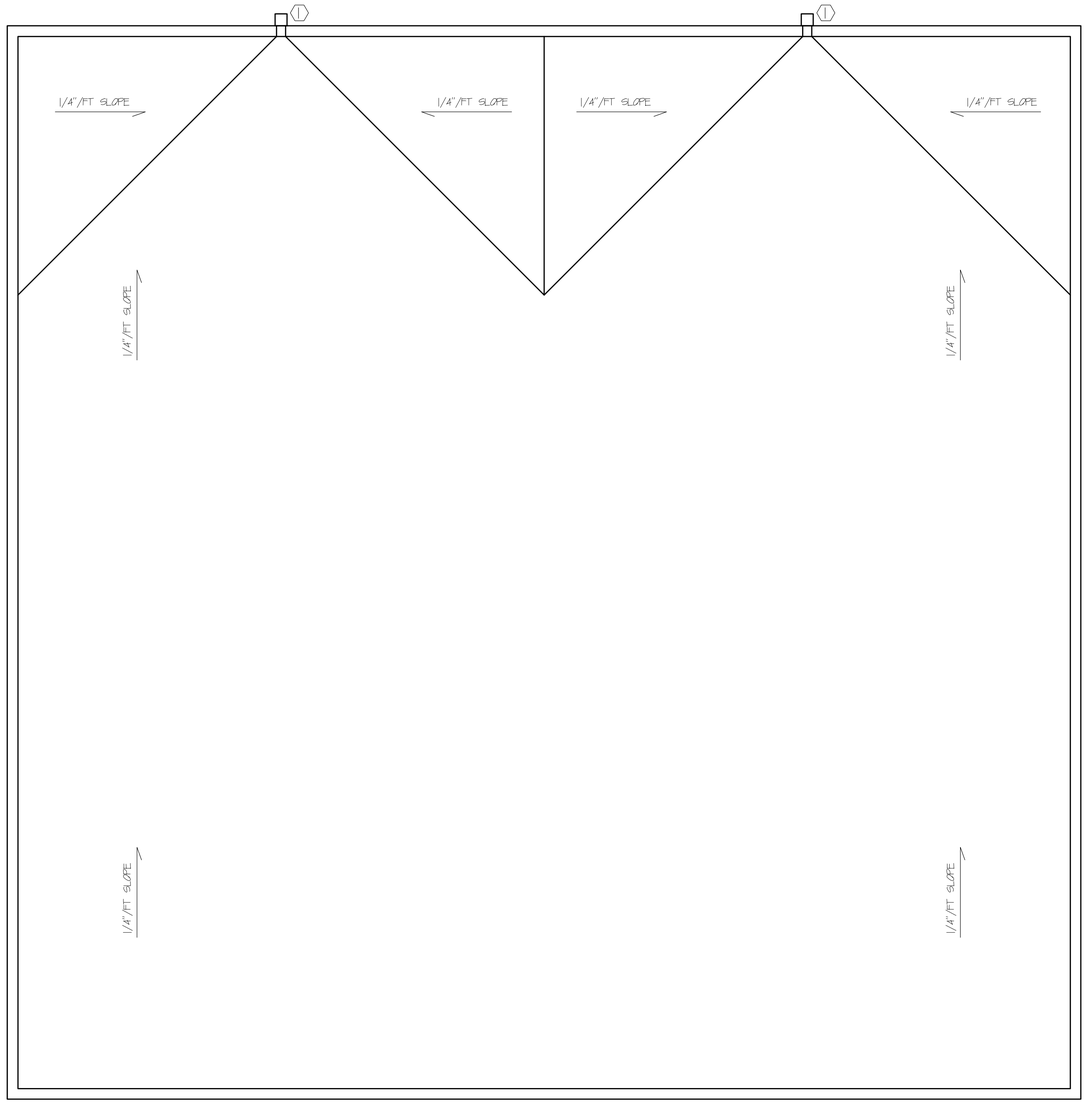
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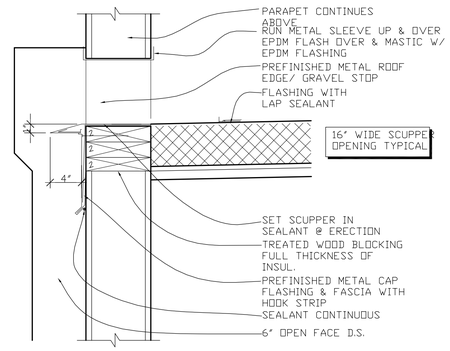
KEY NOTES

1. THRU WALL SCUPPER AND 6" OPEN FACE DOWNSPOUT – PROVICE CONCRETE SPLASH BLOCK AT DISCHARGE

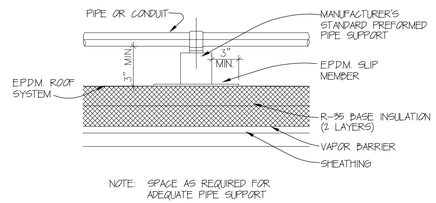
NOTE: COORDINATE PENETRATIONS WITH MECHANICAL



1 A103 ROOF PLAN
SCALE: 1/4" = 1'-0"

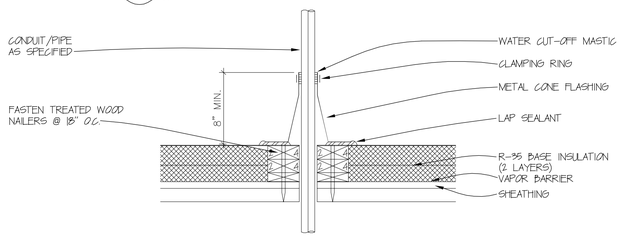


5 A103 THRU WALL SCUPPER
N.T.S.

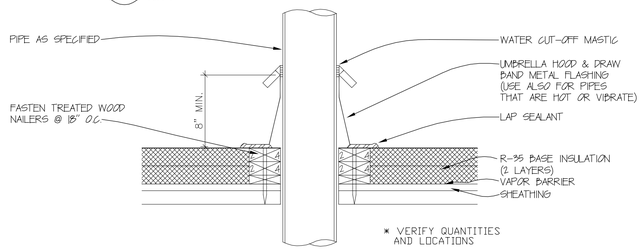


NOTE: SPACE AS REQUIRED FOR ADEQUATE PIPE SUPPORT

4 A103 PIPE SUPPORT
N.T.S.



3 A103 PIPE OPENING (-4" DIA)
N.T.S.



* VERIFY QUANTITIES AND LOCATIONS

2 A103 PIPE OPENING (+4" DIA)
N.T.S.

RINGDAHL Architects, inc
Easton Place
510 22nd Ave. East, Suite 102
Alexandria, MN 56308
ph. (320) 763-9368
fax (320) 762-9063

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ROOF PLAN

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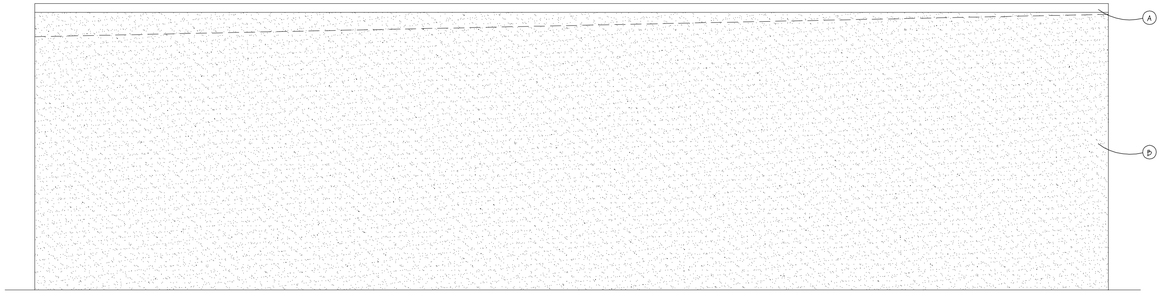
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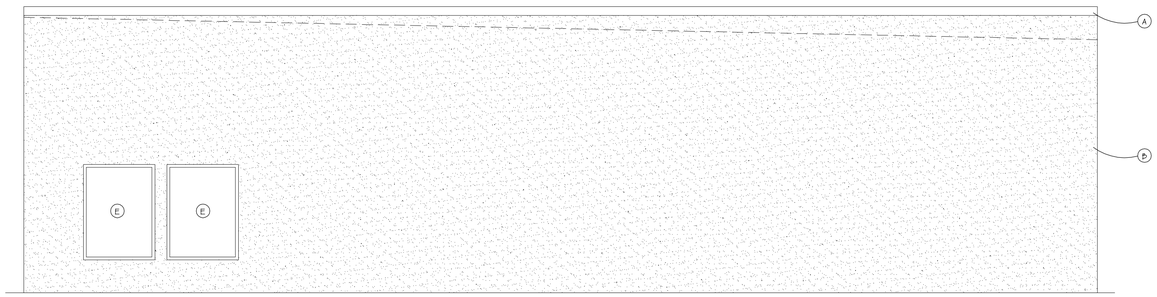
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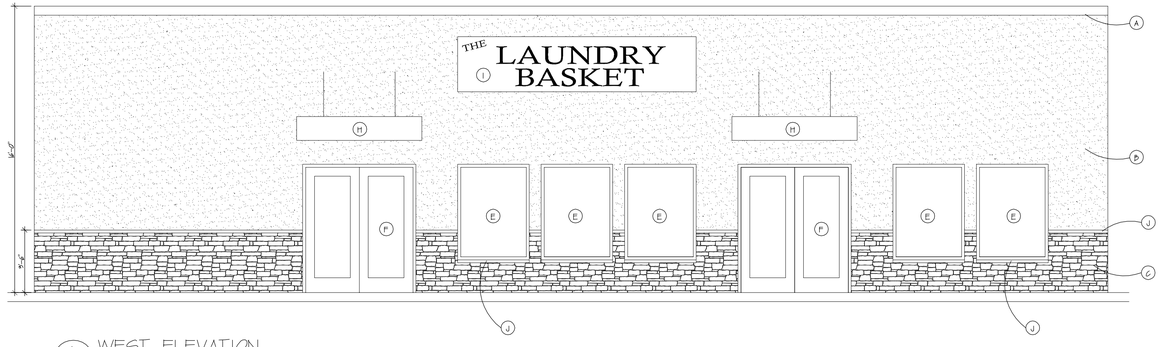
4 A201 NORTH ELEVATION
SCALE: 1/4" = 1'-0"



3 A201 SOUTH ELEVATION
SCALE: 1/4" = 1'-0"



2 A201 EAST ELEVATION
SCALE: 1/4" = 1'-0"



1 A201 WEST ELEVATION
SCALE: 1/4" = 1'-0"

EXTERIOR FINISH SCHEDULE

SYMBOL	BASE MATERIAL	FINISH	REMARKS
A	PREFINISHED METAL PARAPET CAP	PREFINISHED	COLOR BY ARCHITECT
B	EIFS	MATCH SHINGLES	-
C	STONE VENEER	PREFINISHED	COLOR BY ARCHITECT
D	PREFINISHED METAL DS. & SCUPPER	STAIN/PAINT	MATCH SIDING
E	ALUMINUM WINDOW	PREFINISHED	COLOR BY ARCHITECT
F	ALUMINUM ENTRY SYSTEM	PREFINISHED	COLOR BY ARCHITECT
G	HOLLOW METAL DOOR	PREFINISHED	COLOR BY ARCHITECT
H	FABRICATED METAL CANOPY	MATCH NOW COLOR	-
I	SIGNAGE BY OWNER - VERIFY REQUIREMENTS	PREFINISHED SMOOTH	COLOR BY ARCHITECT
J	STONE SILL	PREFINISHED SMOOTH	4" EXPOSURE

NOTES:

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Easton Place
510 22nd Ave. East, Suite 102
Alexandria, MN 56308
ph. (320) 763-9368
fax (320) 762-9063

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EXTERIOR ELEVATIONS

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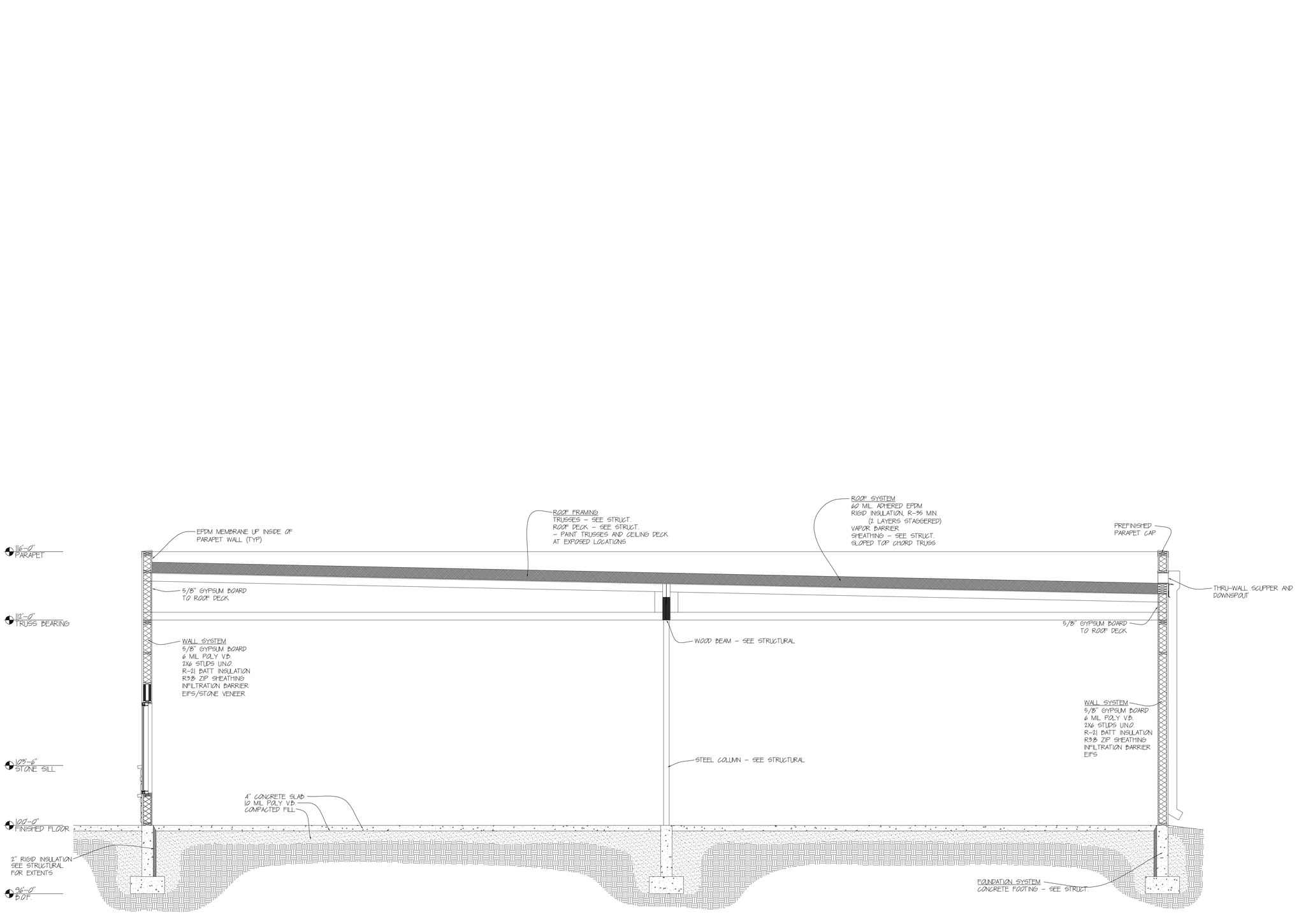
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1 / A301 BUILDING SECTION
SCALE: 3/8" = 1'-0"

BUILDING SECTION

ISSUE FOR:	ISSUE DATE:
PERMIT	10/6/2026

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DATE:	10/6/2026

DRAWING NO.
A301

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STRUCTURAL DRAWING INDEX

SHEET NUMBER	SHEET NAME
S000	TITLE SHEET
S100	GENERAL NOTES
S200	FOUNDATION PLAN
S205	SLAB ON GRADE PLAN
S210	ROOF FRAMING PLAN
S301	FRAMING DETAILS - WOOD
S302	SHEAR WALL AND SHEATHING DETAILS

DRAWINGS SYMBOLS

EXISTING
CONCRETE
FACE BRICK
CONCRETE BLOCK
DIMENSIONAL LUMBER
WOOD BLOCKING
EARTH
COMPACT FILL
LAMINATED VENEER LUMBER (LVL)
STEEL
WELDED WIRE FABRIC
FOOTING STEP
JOIST BOT CHORD EXTENSION
JOIST BOLTED CONNECTION PER OSHA REQUIREMENTS
BEAM OR COL SPLICE LOCATION
BEAM MOMENT CONNECTION
ELEVATION OR WORK POINT
SNOW DRIFT SURCHARGE LOAD
TOP OF SLAB ELEVATION CHANGE (SLOPED)
TOP OR BOTTOM OF SLAB ELEVATION CHANGE (STEPPED)
BEAM CONTINOUS OVER SUPPORT

CCx CONCRETE COLUMN SEE SCHEDULE
CLx CONCRETE LINTEL SEE SCHEDULE
CFx CONTINOUS CONCRETE FOOTING SEE SCHEDULE
CPx CONCRETE PIER SEE SCHEDULE
CWx CIP CONCRETE WALL SEE SCHEDULE
Fx CIP PAD FOOTING SEE SCHEDULE
MCx CMU COLUMN SEE SCHEDULE
LCx LIGHT-GAGE COLUMN SEE SCHEDULE
MJx CMU JAMB SEE SCHEDULE
MLx CMU LINTEL SEE SCHEDULE
MPx CMU PIER SEE SCHEDULE
MWx CMU WALL SEE SCHEDULE
RFx ROUND CONCRETE FOOTING SEE SCHEDULE
LBx Bx LIGHT-GAGE OR WOOD BEAM SEE SCHEDULE
LHx Hx LIGHT-GAGE OR WOOD HEADER SEE SCHEDULE
LWx WWx LIGHT-GAGE OR WOOD WALL SEE SCHEDULE
WCx WOOD COLUMN SEE SCHEDULE
1 KEYNOTE SEE SCHEDULE
LATERAL SHEAR LOAD
SHEAR WALL HOLD DOWNS
CENTERLINE
ARCH/ELEC/MECH PENETRATION
LOW END OF BRACE
LOCATION OF BRACED FRAME
HIGH END OF BRACE

1 / S000 SECTION NUMBER, SHEET NUMBER
SECTION NUMBER, 1 / S000, SHEET NUMBER
1 / S61 ELEVATION NUMBER, SHEET NUMBER
REVISION CLOUD, 1 REVISION MARK
X GRID BUBBLE
DRAWING NAME: X FOUNDATION PLAN, 1/8" = 1'-0"
DRAWING SCALE
DRAWING NUMBER

ABBREVIATIONS

A	
AB	ANCHOR BOLT
ADD'L	ADDITIONAL
ALT	ALTERNATE
ANCH	ANCHOR
ARCH	ARCHITECT(URAL)
B	
BB	BOND BEAM
BLDG	BUILDING
BLK	BLOCK
BLKG	BLOCKING
BM	BEAM
BOT	BOTTOM
BRG	BEARING
BRKT	BRACKET
BTWN	BETWEEN
C	
CANT	CANTILEVER
CFS	COLD-FORMED STEEL
CIP	CAST-IN-PLACE
CJ	CONTROL JOINT
CL	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
COMP	COMPOSITE
CONC	CONCRETE
COND	CONDITION
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
COORD	COORDINATE
CTR	CENTER
CTR'D	CENTERED
D	
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DEFL	DEFLECTION
DF	DOUGLAS-FIR
DIA	DIAMETER
DIAG	DIAGNOL
DIM	DIMENSION
DL	DEAD LOAD
DTL	DETAIL
DWG(S)	DRAWING(S)
DWL(S)	DOWELS
E	
E	EAST
(E)	EXISTING
EA	EACH
EF	EACH FACE
EL	EARTHQUAKE LOAD
EW	EACH WAY
EJ	EXPANSION JOINT
ELEC	ELECTRIC
EL OR ELEV	ELEVATION
EMBED	EMBEDMENT
EOR	ENGINEER OF RECORD
EQ	EQUAL
EXP	EXPANSION
EXT	EXTEND OR EXTERIOR
F	
FAB	FABRICATE(OR)
FD	FLOOR DRAIN
FLG	FLANGE
FLR	FLOOR
FNDN	FOUNDATION
FT	FEET
FTG	FOOTING
FV	FIELD VERIFY
G	
GA	GAGE OR GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GL	GLUE LAMINATED WOOD
H	
HORIZ	HORIZONTAL
HF	HEM-FIR
HK	HOOK
HL	HYDROSTATIC LOAD
HS	HEADED STUD(S)
HSS	HOLLOW STRUCTURAL SHAPE
I	
ID	INSIDE DIAMETER
IF	INSIDE FACE
IN	INCH
INFO	INFORMATION
INSUL	INSULATION
INT	INTERIOR
J	
JBE	JOIST BEARING ELEVATION
JST	JOIST
JT	JOINT
K	
K OR KIP(S)	KIP = 1,000 LB
KO	KNOCK-OUT
KLF	KIPS PER LINEAL FOOT
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	
LG	LIGHT GUAGE STEEL
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSH	LONG SIDE HORIZONTAL
LSL	LAMINATED STRAND LUMBER
LSV	LONG SIDE VERTICAL
LVL	LAMINATED VENEER LUMBER
LW	LIGHT-WEIGHT
M	
M	MOMENT
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MECH	MECHANICAL
MEP	MECH, ELECT & PLUMBING
MEZZ	MEZZANINE
MFG	MANUFACTURE(R)
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
N	
N	NORTH
NF	NEAR FACE
NO	NUMBER
NOM	NOMINAL
NTS	NOT TO SCALE
O	
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OH	OVERHEAD
OPNG	OPENING
OPP	OPPOSITE
P	
PC	PRECAST CONCRETE
PCF	POUNDS PER CUBIC FOOT
PEMB	PRE-ENGINEERED METAL BUILDING
PERIM	PERIMETER
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PROJ	PROJECT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSL	PARALLEL STRAND LUMBER
PT	POST-TENSION
PW	PRECAST WALL
Q	
QTY	QUANTITY
R	
RAD	RADIUS
RD	ROOF DRAIN
REINF	REINFORCE(D), (ING)
REQ'D	REQUIRED
REV	REVERSE
RL	RAIN LOAD
RLL	ROOF LIVE LOAD
S	
S	SOUTH
SCHED	SCHEDULE
SD	SEE DETAIL
SEC	SECTION
SHT	SHEET
SIM	SIMILAR
SIPs	STRUCTURAL INSULATED PANELS
SL	SNOW LOAD
SOG	SLAB-ON-GRADE
SPA	SPACE(S)
SPEC(S)	SPECIFICATION(S)
SQ	SQUARE
STAGG	STAGGER
STD	STANDARD
STL	STEEL
STRUCT	STRUCTURAL
SUPPORT	SUPPORT
SP	SOUTHERN PINE
SPF	SPRUCE PINE-FIR
T	
T/	TOP OF ELEMENT
T&B	TOP AND BOTTOM
TBE	TOP OF BEAM ELEVATION
TDE	TOP OF DECK ELEVATION
TEMP	TEMPORARY
TFE	TOP OF FOOTING ELEVATION
THRU	THROUGH
TOS	TOP OF STEEL
TPC	TOP OF PILE CAP ELEVATION
TPE	TOP OF PIER ELEVATION
TSE	TOP OF SLAB ELEVATION
TWE	TOP OF WALL ELEVATION
TYP	TYPICAL
U	
UN	UNLESS NOTED
UNO	UNLESS NOTED OTHERWISE
V	
V	SHEAR
VER	VERIFY
VERT	VERTICAL
VIF	VERIFY IN FIELD
W	
W	WEST
W/	WITH
W/O	WITHOUT
WL	WIND LOAD
WP	WORK POINT
WWF	WELDED WIRE FABRIC
WWR	WELDED WIRE REINFORCEMENT

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TITLE SHEET

DRAWING NO.

S000

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GENERAL NOTES

I. DESIGN DATA

A. BUILDING CODE

1. INTERNATIONAL BUILDING CODE (IBC) 2018 - AS AMENDED BY THE STATE OF MINNESOTA.

B. DESIGN LOADS/DESIGN CRITERIA

1. DEFLECTION CRITERIA SHALL MEET THE FOLLOWING MINIMUM REQUIREMENTS:
   A. DEFLECTION LIMITS INDICATED IN TABLE 1604.3 OF THE IBC.
2. WIND LOAD:
   A. BASIC WIND SPEED (3-SECOND GUST)
      a. V_{ULT} = 115 MPH. RISK CATEGORY II
      b. V_{ASD} = 90 MPH, IMPORTANCE FACTOR, I = 1.0
      c. EXPOSURE C
      d. INTERNAL PRESSURE COEFFICIENT, GC_{PI} = +/- 0.18
3. SEISMIC DESIGN DATA:
   a. SEISMIC DESIGN NOT REQUIRED PER MSBC 1305.0011 SUBPARAGRAPH 4.
4. ROOF LOADS:
   A. LIVE LOAD (RLL): 20 PSF*
   B. DESIGN DEAD LOAD: 20 PSF (ADHERED)
5. ROOF SNOW LOADS:
   A. GROUND SNOW LOAD, P_g: 50 PSF
   B. FLAT ROOF SNOW LOAD, P_f: 35 PSF**
   C. SNOW EXPOSURE FACTOR, C_e: 1.0 PSF
   D. SNOW LOAD IMPORTANCE FACTOR, I_s: 1.0
   E. THERMAL FACTOR, C_t: 1.0
   * REDUCED PER IBC SECTION 1607.11
   ** PLUS SNOW ACCUMULATION AS REQUIRED BY IBC CHAPTER 16, SECTION 1608.

A. DEFERRED STRUCTURAL SUBMITTALS

1. DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF PERMIT APPLICATION BUT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO INSTALLATION. THE FOLLOWING ITEMS ARE TO BE ISSUED AS DEFERRED SUBMITTALS:
   a. TEMPORARY SHORING
   b. TEMPORARY EARTH RETENTION SYSTEMS
   c. METAL PLATED CONNECTED WOOD TRUSSES, PLYWOOD WEB JOISTS AND CONNECTIONS
2. INCLUDE DESIGN AND CALCULATIONS PREPARED AND CERTIFIED BY AN ENGINEER LICENSED TO PRACTICE IN THE STATE IN WHICH THE PROJECT IS LOCATED AS A PART OF THE SUBMITTAL DOCUMENTS. PROVIDE DEFERRED SUBMITTALS A MINIMUM 30 DAYS PRIOR TO INSTALLATION, AND AFTER CONTRACTOR'S REVIEW, TO THE ARCHITECT/ENGINEER FOR REVIEW FOR GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS. SUBMIT TO BUILDING OFFICIAL FOR REVIEW AND APPROVAL AFTER ARCHITECT/ENGINEER'S REVIEW. DEFERRED SUBMITTAL ITEMS ARE NOT TO BE INSTALLED UNTIL AFTER THE SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND APPROVED BY BUILDING OFFICIAL.

B. ALTERNATE DESIGNS

1. ALTERNATE STRUCTURAL SYSTEMS AND DETAILS WILL ONLY BE CONSIDERED PROVIDED THEY ARE SUBMITTED WITH CALCULATIONS CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. THE CALCULATIONS MUST SHOW THE EQUIVALENCY OF THE ALTERNATE. ACCEPTANCE OF THE ALTERNATE BY THE ENGINEER OF RECORD MUST BE IN WRITING.

C. EXPANSION JOINTS

1. EXPANSION JOINTS ARE NOTED ON THE DRAWINGS. NO CONNECTION SHALL BE MADE ACROSS THESE JOINTS UNLESS NOTED OTHERWISE..

D. FUTURE EXPANSION

1. THIS PROJECT IS NOT DESIGNED FOR FUTURE EXPANSION.

E. GENERAL CONSTRUCTION NOTES:

1. IN ALL CASES WHERE A CONFLICT MAY OCCUR, SUCH AS BETWEEN REQUIREMENTS IN THE SPECIFICATION AND REQUIREMENTS ON THE DRAWINGS, THE STRUCTURAL ENGINEER OF RECORD SHALL BE NOTIFIED IMMEDIATELY IN WRITING AND THE STRUCTURAL ENGINEER OF RECORD SHALL INTERPRET THE INTENT OF THE CONTRACT DRAWING.
2. IN NO CASE, SHALL WORKING DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THE STRUCTURAL DRAWINGS.
3. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND CONDITIONS AT THE JOBSITE AND TO CROSS CHECK ALL DETAILS AND DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH RELATED REQUIREMENTS ON THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND CIVIL DRAWINGS. NOTIFY THE STRUCTURAL ENGINEER OF RECORD ANY DISCREPANCIES PRIOR TO COMMENCING WORK.
4. VERIFY SIZE AND LOCATIONS OF ALL OPENINGS WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. STRUCTURAL DRAWINGS DO NOT NECESSARILY SHOW ALL OPENINGS. PLACE OPENINGS IN FLOOR AND ROOF NOT SHOWN ON STRUCTURAL DRAWINGS BETWEEN STRUCTURAL MEMBERS. NOTIFY ENGINEER OF RECORD OPENINGS LARGER THAT 12" IN ANY DIMENSION ARE ADDED.
5. IN EXISTING FACILITIES, ALL EXISTING CONDITIONS MUST BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. ANY EXISTING CONDITIONS THAT DIFFER FROM THOSE SHOWN ON THE STRUCTURAL DRAWINGS MUST BE BROUGHT TO THE IMMEDIATE ATTENTION TO THE STRUCTURAL ENGINEER (IN WRITING).

F. TEMPORARY BRACING:

1. THE DESIGN OF STUD WALL FRAMING IS FOR COMPLETED, SHEATHED CONDITION. INCLUDING PROPER FASTENING, THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF TEMPORARY BRACING. IF THE CONTRACTOR ELECTS TO PLACE WALL SHEATHING OR FLOOR TOPPING ON ANY LEVELS ABOVE STUD FRAMING THAT DO NOT YET HAVE SHEATHING, ONE OF THE FOLLOWING ARE REQUIRED PRIOR TO SUCH PLACEMENT:
   A. PROVIDE BLOCKING THAT MATCHES THE SIZE AND SPECIES OF STUDS AT MID-HEIGHT OF ALL LOAD-BEARING STUD WALLS AT THE BOTTOM THREE LEVELS IN FIVE LEVEL WOOD STRUCTURES AND AT THE BOTTOM TWO LEVELES IN FOUR LEVEL WOOD STRUCTURES, EXCEPT THAT WALLS WITH DOUBLED OR TRIPPLED STUDS FASTENED WITH 10d NAILS AT 8" OC STAGGERED EACH FACE DO NOT REQUIRE BLOCKING. BLOCKING REQUIRED BY THE SHEAR WALL SCHEDULE IS NOT ELIMINATED BY THE ABOVE NAILING OPTION.
   B. RETAIN A SPECIALTY STRUCTURAL ENGINEER TO EVALUATE THE ADEQUACY OF TEMPORARY UNBRACED STUD WALL CONDITIONS, DESIGN RELATED BRACING FOR THE CONDITIONS, AND SUBMIT CERTIFIED DRAWINGS FOR RECORD ONLY.
2. PROVIDE TEMPORARY BRACING FOR ALL WALLS (CONCRETE, MASONRY, COLD FORMED STEEL OR WOOD) UNTIL THEY ARE OF ADEQUATE DESIGN STRENGTH AND ARE PROPERLY ANCHORED IN FINAL FORM.

G. REFERENCE STANDARDS – REFER TO IBC CHAPTER 35 FOR ALL REFERENCE STANDARDS.

II. SITE WORK

A. ASSUMED GEOTECHNICAL PROPERTIES

1. FOUNDATIONS ARE DESIGNED BASED ON THE ASSUMED DESIGN INFORMATION OUTLINED IN THIS SECTION. THE CONTRACTOR/OWNER SHALL RETAIN THE SERVICES OF A GEOTECHNICAL ENGINEER, LICENSED IN THE STATE OF THE PROJECT, TO VISIT THE SITE AND CERTIFY IN WRITING THAT THE FOLLOWING ASSUMED INFORMATION IS CORRECT.
2. ASSUMED DESIGN NET SOIL BEARING CAPACITY IS AS FOLLOWS:
   A. SPREAD FOOTINGS: 1,500 PSF
   B. STRIP FOOTINGS: 1,500 PSF
3. MINIMUM DEPTH FROM EXTERIOR GRADE TO BOTTOM OF BUILDING PERIMETER FOOTINGS SHALL BE 42".
4. FOUNDATIONS SHALL NOT BE CAST ON FROZEN SOILS. IF IT IS ANTICIPATED THAT THE SOILS BELOW THE FOUNDATIONS MAY FREEZE, CONTRACTOR SHALL SUBMIT A FROST PROTECTION PLAN IN WRITING TO ENGINEER OF RECORD.
5. THE GEOTECHNICAL ENGINEER SHALL VERIFY THAT THE FOOTING ELEVATIONS SHOWN THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF LOCATIONS THAT DO NOT SATISFY THOSE REQUIREMENTS. IN THOSE CASES, THE GRADE MAY BE ELEVATED WITH COMPACTED MATERIAL APPROVED BY GEOTECHNICAL ENGINEER OR THE FOOTING DEPTH MAY BE INCREASED WITH PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.

B. **BACKFILLING**

1. BACKFILL AND MATERIALS BEHIND FOUNDATION AND RETAINING WALLS SHALL BE AS OUTLINED IN THE GEOTECHNICAL REPORT.
2. DO NOT BACKFILL OR COMPACT EARTH AGAINST WALLS RETAINING EARTH UNTIL SUPPORTING SLABS HAVE REACHED 75% OF THEIR DESIGN STRENGTH OR ADEQUATE BRACING IS IN PLACE. PROVIDE BRACING DRAWINGS DESIGNED AND CERTIFIED BY AN ENGINEER LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED FOR RECORD ONLY.
3. BACKFILL EVENLY ON BOTH SIDES OF FOUNDATION WALLS TO PREVENT OVERTURNING OR LATERAL WALL MOVEMENT.

III. CONCRETE

A. CONCRETE MATERIAL PROPERTIES

1. CONCRETE PROPERTIES:

	STRENGTH (f'c @ 28 DAYS)	MAX W/C RATIO
FOOTINGS	3,000 PSI	
PIERS	4,000 PSI	
INTERIOR SLAB ON GRADE	4,000 PSI	
EXTERIOR CONCRETE (SLAB ON GRADE, PIERS, SITE WALLS, AND RETAINING WALLS)	5,000 PSI	.40

2. CYLINDER TESTING SHALL BE COMPLETED PER ACI-318, SECTION 26.12. TESTING REPORTS SHALL BE PROVIDED TO THE OWNER AND ENGINEER OF RECORD AT A MINIMUM. PREFERRED DELIVERY METHOD IS VIA E-MAIL.
3. ALL EXTERIOR CONCRETE, PERMANENTLY EXPOSED TO WEATHER (DOES NOT APPLY TO BURIED FOUNDATIONS), SHALL BE AIR ENTRAINED TO GIVE THE CONCRETE AN AIR CONTENT OF 6% +/- 1.5% BY VOLUME. NATURALLY OCCURRING AIR CONTENT SHALL NOT EXCEED 3% FOR NON-AIR ENTRAINED MIXES.
4. CONCRETE MIX DESIGNS & SUPPORTIVE DATA MUST BE SUBMITTED FOR APPROVAL PER ACI 318 SECTION 26.4.3.1(B), AND ACI-301, SECTION 1.5.

B. REINFORCING MATERIAL PROPERTIES

1. REINFORCING PROPERTIES:

	f_y (KSI)	ASTM
ALL BARS, UNO	60	A615
TIES & STIRRUPS	60	A615
WELDABLE REBAR	60	A706/AWSD1.4.

2. DETAIL, FABRICATE, AND PLACE REINFORCING IN ACCORDANCE WITH CRSI "MANUAL OF STANDARD PRACTICE", LATEST ADDITION AND ACI 117.
3. EPOXY COATED REINFORCING REQUIRED WHERE REINFORCED CONCRETE IS EXPOSED TO MOISTURE AND CHLORIDES, SUCH AS DE-ICING SALTS. WHERE EPOXY COATED REINFORCING IS REQUIRED, ALL CHAIRS, SLAB BOLSTERS, SUPPORT BARS, AND SPACERS SHALL BE PLASTIC OR EPOXY COATED.
4. EPOXY COATING FOR REINFORCING SHALL CONFORM TO ASTM A775 AND ACI 301 SECTION 3.2. REPAIR COATING DAMAGE IN ACCORDANCE WITH ASTM D3963 AND PATCHING MATERIAL MANUFACTURER'S RECOMMENDATIONS.

C. CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH IBC CHAPTER 19 AND ACI 318. PERFORM
2. ALL REINFORCING SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH THE LATEST EDITION OF THE "CRSI - MANUAL OF STANDARD PRACTICE" AND ACI 117. SUBMIT SHOP DRAWINGS FOLLOWING THE RECOMMENDED PRACTICES OF ACI 315 INDICATING REINFORCING SIZE, LENGTH, LOCATION, SHAPE AND SPACING OF BARS.
3. PROVIDE STANDARD HOOKS WHERE HOOK LENGTHS ARE NOT SPECIFIED.
4. PER ACI 26.6.2.2(A) and 26.10.2(B), ALL REINFORCEMENT SHALL BE PLACED AND SUPPORTED PRIOR TO PLACING CONCRETE. "WET STICKING" OF REBAR, INCLUDING DOWELS IS PROHIBITED.
5. SLEEVES EMBEDDED IN SLABS AND WALLS SHALL BE LOCATED CLEAR BETWEEN REINFORCING BARS AND SHALL MAINTAIN CLEAR SPACING EQUAL TO THE DIAMETER OF THE LARGEST SLEEVE IN ANY DIRECTION. SLEEVE GROUPS THAT DO NOT COMPLY WITH THE ABOVE REQUIREMENTS SHALL BE CONSIDERED AS AN OPENING AND REINFORCED PER NOTE #5 BELOW.
6. UNLESS NOTED OTHERWISE ON THE DRAWINGS: PROVIDE EXTRA REINFORCING ON ALL SIDES OF ALL MISCELLANEOUS WALL AND SLAB OPENINGS EQUAL TO ONE HALF THE INTERRUPTED REINFORCING BARS EACH SIDE BUT NOT LESS THAN 2 - #5 FOR EACH LAYER OF REINFORCEMENT. EXTEND BARS CLASS 'B' LAP LENGTH BUT NOT LESS THAN 2 FEET BEYOND EDGE OF OPENINGS. PROVIDE 2 - #4x4'-0" DIAGONAL BARS AT EACH CORNER FOR EACH LAYER OF REINFORCEMENT.
7. PROVIDE A ¾" CHAMFER ON ALL EXPOSED CORNERS OF CONCRETE.
8. PROVIDE ISOLATION JOINTS AROUND COLUMNS AT SLAB ON GRADE AREAS.
9. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR CONCRETE FINISH REQUIREMENTS.
10. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

	CONCRETE COVER (IN)
CONCRETE CAST AGAINST & PERMANENTLY EXPOSED TO EARTH	3
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THRU #18 BARS	2
#5 & SMALLER BARS	1 ½
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS & WALLS:	
#14 & #18 BARS	1 ½
#11 AND SMALLER BARS	¾
BEAMS & COLUMNS:	
PRIMARY REINFORCEMENT, STIRRUPS, & TIES	1 ½

IV. STEEL

A. STEEL MATERIAL PROPERTIES

1. STEEL PROPERTIES:

	STRENGTH (PSI)	ASTM
OTHER STRUCT. SHAPES & PLATES, ETC.	36,000	A36
ANCHOR BOLTS, UNO	36,000	F1554
WELDING ELECTRODES	E70XX	A233
STRUCTURAL PIPE	35,000	A53 GRADE B

B. STRUCTURAL STEEL

1. STRUCTURAL STEEL DESIGN & CONSTRUCTION SHALL CONFORM TO IBC CHAPTER 22, AISC "LOAD & RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" & AISC "CODE OF STANDARD PRACTICE," APPLY UNO.
2. STRUCTURAL STEEL SUPPLIER SHALL SUBMIT SHOP DRAWINGS FOR ALL MATERIAL SUPPLIED. IN ADDITION, THE STRUCTURAL STEEL SUPPLIER SHALL SUBMIT DRAWINGS AND CALCULATIONS CERTIFIED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT FOR ALL STAIRS, LADDERS, RAILINGS, CAP PLATES, BEARING PLATES, BASE PLATES, STIFFENERS, SPLICES, CONNECTIONS AND ANY OTHER COMPONENTS DESIGNED BY THE SUPPLIER.
3. PROVIDE PROTECTIVE ASPHALTIC COATING OR EQUAL AROUND STRUCTURAL STEEL BELOW GRADE.
4. GROUTING UNDER COLUMN BASE PLATES SHALL BE ACCOMPLISHED WITH NONMETALLIC, SHRINK RESISTING GROUT, ASTM C1103 WITH A MINIMUM COMPRESSIVE STRENGTH OF 7,000 PSI IN ACCORDANCE WITH ACI 351.1.
5. ALL STRUCTURAL STEEL MEMBERS SHALL BE CLEANED TO SSPC-SP3, AND PRIMED WITH STANDARD SHOP PRIMER WITH THE EXCEPTION OF AREA TO BE FIELD WELDED, EMBEDDED IN CONCRETE, RECEIVE FIELD FIRE PROTECTION, OR WITH SLIP CRITICAL CONNECTIONS.

V. WOOD / TIMBER CONSTRUCTION

A. ROUGH CARPENTRY

1. STRUCTURAL LUMBER SHALL BE DESIGNED & CONSTRUCTED IN ACCORDANCE WITH IBC CHAPTER 23 & THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION.
2. THIS STRUCTURE IS A NON-SELF-SUPPORTING WOOD FRAME REQUIRING INTERACTION WITH OTHER ELEMENTS TO PROVIDE THE REQUIRED STABILITY. THE FRAMER SHALL PROVIDE TEMPORARY BRACING UNTIL FINAL STABILITY IS PROVIDED.
3. ALL DIMENSION LUMBER SHALL BE GRADE STAMPED.
4. ALL MEMBER SIZES SHOWN ON THE DRAWINGS ARE NOMINAL DIMENSIONS. DRESSED SIZES PUBLISHED IN AMERICAN SOFTWOOD LUMBER STANDARDS PS 20 SHALL BE ACCEPTED AS MINIMUM NET SIZES CONFORMING TO SUCH NOMINAL SIZES.
5. ALL BEAMS & JOISTS NOT BEARING ON SUPPORTING MEMBERS SHALL BE FRAMED WITH PREFABRICATED METAL JOIST HANGERS OF REQUIRED CAPACITY. ALL PREFABRICATED METAL HARDWARE IS BY SIMPSON STRONG-TIE COMPANY OR APPROVED EQUAL. AND SHALL BE DESIGNED AND PROVIDED BY THE WOOD MATERIAL SUPPLIER.
6. WOOD JOISTS SHALL BEAR ON THE FULL WIDTH OF SUPPORTING MEMBERS UNO.
7. LAG SCREWS ARE TO BE INSTALLED IN PRE-DRILLED HOLES.
8. BOLTED CONNECTIONS ARE TO BE INSTALLED THRU PRE-DRILLED HOLES NO LARGER THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTS ARE TO BE STANDARD HEX FULL DIAMETER BOLTS COMPLYING WITH THE SAE J429 WITH A FASTENER YIELD STRENGTH OF 45,000 PSI. STANDARD WASHERS ARE TO BE USED UNDER ALL NUTS AND BOLT HEADS.
9. FASTENERS AND CONNECTIONS USED IN PRESERVATIVE TREATED AND FIRE-RETARDANT WOOD SHALL BE HOT DIPPED GALVANIZED (ASTM A153 CLASS D).
10. NAILING SHALL BE IN ACCORDANCE WITH WOOD FASTENING SCHEDULE ON SHEET --- OR AS SHOWN ON THE DRAWINGS. SPACE NAILS TO AVOID SPLITTING. ALL NAILS ARE COMMON UNO.
11. ALL HEADERS AND BEAMS SHALL HAVE KING AND JACK STUDS AT BEARING SUPPORTS AS SHOWN ON THE DRAWINGS.
12. ENGINEERED UPLIFT CONNECTIONS SHALL BE PROVIDED AT ALL ROOF MEMBER CONNECTIONS TO WALL TOP PLATES.
13. SLEEPERS AND SILLS BEARING ON CONCRETE OR MASONRY AND PERMANENT COLUMNS SUPPORTED BY FOOTINGS IN CONTACT WITH EARTH SHALL BE PRESERVATIVE TREATED.
14. WHERE PENNY WEIGHTS ARE INDICATED ON THE DRAWINGS, THE NAILS SHALL COMPLY WITH THE MINIMUM DIAMETERS INDICATED BELOW:

PENNY WEIGHT	MIN. SHANK DIAMETER (IN)	MIN. LENGTH (IN)
8D	0.131	2 1/2
10D	0.148	3
16D	0.162	3 1/2

15. NAILS AND STAPLES USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL CONFORM TO ASTM F 1667 AND SHALL HAVE THE FOLLOWING MINIMUM BENDING YIELD STRENGTHS:

SHANK DIAMETER (IN)	BENDING YIELD STRENGTH
0.178" – 0.254"	80 KSI
0.143" – 0.177"	90 KSI
0.099" – 0.142" (16D & SMALLER)	100 KSI

16. LUMBER USED SHALL BE KILN DRIED TO A MOISTURE CONTENT NOT EXCEEDING 19% COMPLYING WITH THE FOLLOWING MINIMUM DESIGN PARAMETERS UNLESS NOTED OTHERWISE (ALTERNATIVE MATERIAL WITH BETTER DESIGN VALUES MAY BE USED WITH PRIOR APPROVAL).
17. PROVIDE ALLOWANCE IN STUDS AND SHEATHING AS REQUIRED FOR PLUMBING PIPES, ETC. DUE TO OVERALL WOOD SHRINKAGE ABOVE EACH LEVEL AS FOLLOWS:
    1ST LEVEL ABOVE PLANK/SLAB: UP TO 1/8"
    2ND LEVEL ABOVE PLANK/SLAB: UP TO ½"
    3RD LEVEL ABOVE PLANK/SLAB: UP TO 7/8"
    4TH LEVEL ABOVE PLANK/SLAB: UP TO 1 ¼"
    5TH LEVEL ABOVE PLANK/SLAB: UP TO 1 5/8"

DIMENSIONAL LUMBER			MINIMUM DESIGN PROPERTIES				
COMPONENT	WOOD SPECIES	GRADE	E	F_b	F_v	$F_{C-\perp}$	F_{C-PERP}
BEARING WALL STUDS	SPRUCE-PINE-FIR (SPF)	NO. 1 / NO. 2	1,400,000 PSI	875 PSI	135 PSI	1,150 PSI	425 PSI
TOP PLATES & SOLE PLATE ON WOOD BASE	SPRUCE-PINE-FIR (SPF)	NO. 1 / NO. 2	1,400,000 PSI	875 PSI	135 PSI	1,150 PSI	425 PSI
BEAMS / HEADERS	HEM-FIR (HF)	NO. 2	1,300,000 PSI	850 PSI	170 PSI	1,300 PSI	405 PSI
POSTS (5x5 AND LARGER)	SOUTHERN PINE (SP/SYP)	NO. 1	PER SPIB GRADING RULES AND AWC NDS				
SOLE PLATE IN CONTACT WITH CONCRETE, MASONRY, STEEL, & SOIL	SOUTHERN PINE (SP/SYP)	NO. 2	PER SPIB GRADING RULES AND AWC NDS				

B. ENGINEERED WOOD PRODUCTS

1. ENGINEERED WOOD SHALL BE DESIGNED & CONSTRUCTED IN ACCORDANCE W/ IBC CHAPTER 23 & THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION.
2. ENGINEERED WOOD SUPPLIER SHALL PROVIDE ALL HANGERS, HARDWARE AND ACCESSORIES REQUIRED FOR INSTALLATION.

ENGINEERED LUMBER		MINIMUM DESIGN PROPERTIES				
COMPONENT	WOOD SPECIES	E	F_b	F_v	$F_{C-\perp}$	F_{C-PERP}
BEAMS / HEADERS	LVL	2,000,000 PSI	2,600 PSI	285 PSI	2,510 PSI	750 PSI
RIM BOARD	LSL (LVL ALTERNATE)	1,300,000 PSI	1,700 PSI	425 PSI	1,835 PSI	710 PSI
POSTS / COLUMNS	PSL	1,800,000 PSI	2,400 PSI	190 PSI	2,500 PSI	425 PSI
TALL WALL STUDS	LSL	1,500,000 PSI	2,250 PSI	505 PSI	2,105 PSI	860 PSI

B. OPEN WEB WOOD TRUSSES

1. THE DESIGN AND FABRICATION OF ALL TRUSSES SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND ITS FASTENINGS BY NATIONAL FOREST PRODUCTS ASSOCIATION AND NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION BY THE TRUSS PLATE INSTITUTE.
2. WOOD TRUSS SUPPLIER SHALL DESIGN AND SUPPLY ALL MATERIAL, CONNECTORS, AND ACCESSORIES REQUIRED FOR INSTALLATION INCLUDING, BUT NOT LIMITED TO HANGERS, WALL CONNECTIONS, BLOCKING, WEB STIFFENERS, AND RIM BOARDS TO PROVIDE A COMPLETE INSTALLATION.
3. WOOD TRUSS SUPPLIER SHALL PRODUCE A TRUSS SUBMITTAL PACKAGE INCLUDING THE INFORMATION REQUIRED BY IBC SECTION 2303.4.
4. TRUSS SUBMITTAL PACKAGE SHALL BE SIGNED AND SEALED BY AN ENGINEER LICENSED IN THE STATE OF THE PROJECT.
5. PROVIDE STANDARD CAMBER FOR DEAD LOAD AT ALL ROOF AND FLOOR TRUSSES.
6. NOTCHES SHALL NOT BE CUT IN WOOD JOISTS.
7. WOOD TRUSS SUPPLIER SHALL ACCOUNT FOR THE DEAD LOADS ASSOCIATED WITH FIXED ROOFTOP EQUIPMENT. PLACEMENT OF MECHANICAL UNITS & HANGERS SUPPORTED BY ROOF TRUSSES IS SUBJECT TO THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.

C. WOOD STRUCTURAL SHEATHING

1. GENERAL
   A. PLYWOOD SHEATHING SHALL CONFORM TO IBC SECTION 2303.1.4 AND SHALL BE GRADE STAMPED BY APA. PROVIDED THE THICKNESS AND GRADE SHOWN ON THE DRAWINGS.
   B. NAIL SIZE AND PATTERN SHALL BE AS INDICATED ON THE DRAWINGS.
   C. PLYWOOD SHALL HAVE A MINIMUM OF 4 PLIES.
   D. ORIENTED STRAND BOARD (OSB) IS AN ACCEPTABLE ALTERNATE WITH PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
2. FLOOR AND ROOF SHEATHING – HORIZONTAL USE
   A. PANEL FLOOR SHEATHING SHALL BE APA EXPOSURE 1, TONGUE AND GROOVE PLYWOOD AND AS INDICATED IN THE FRAMING PLAN NOTES.
   B. PANEL ROOF SHEATHING SHALL BE APA EXPOSURE 1, FLAT PLYWOOD AND AS INDICATED IN THE FRAMING PLAN NOTES.
   C. SHEATHING SHALL BE ORIENTED WITH THE STRONG AXIS PERPENDICULAR TO SUPPORTS.
   D. ALL WOOD PANELS SHALL BE CONTINUOUS OVER A MINIMUM OF TWO SPANS WITH A MINIMUM PANEL WIDTH NOT LESS THAN 2'-0" UNLESS ALL PANEL EDGES ARE BLOCKED.
   E. FASTENERS SHALL NOT BE LOCATED CLOSER THAN 3/8" FROM PANEL EDGES.
3. WALL SHEATHING/SHEAR WALL PANELS – VERTICAL USE
   A. SEE THE DRAWINGS FOR SHEAR WALL SHEATHING THICKNESS, TYPE AND GRADE.
   B. REFER TO ARCHITECTURAL DRAWINGS FOR NON-SHEAR WALL SHEATHING TYPE.
   C. SEE THE DRAWINGS FOR SPECIFIC SHEAR WALL FASTENING REQUIREMENTS.
   D. SHEATHING NAILING SHALL BE IN ACCORDANCE WITH WOOD FASTENING SCHEDULE ON SHEET S302. SEE WOOD SHEAR WALL SCHEDULE ON SHEET S302 FOR ADDITIONAL INFORMATION. SPACE NAILS TO AVOID SPLITTING. ALL NAILS ARE COMMON UNO.
   E. WALL SHEATHING MAY BE ORIENTED EITHER HORIZONTALLY OR VERTICALLY. SEE SHEAR WALL SCHEDULE FOR MIN BLOCKING REQUIREMENTS.

DESIGN TREE
engineering + land surveying
Corporate Office:
120 17th Ave W Alexandria, MN 56308
888-216-1916

I HEREBY CERTIFY THAT THIS PLAN SPECIFICATION, OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

PRINTED NAME: JOE A NICHOLSON

DATE: 9/18/2026 LICENSE #: 44104

WILLMAR LAUNDRY

WILLMAR, MN

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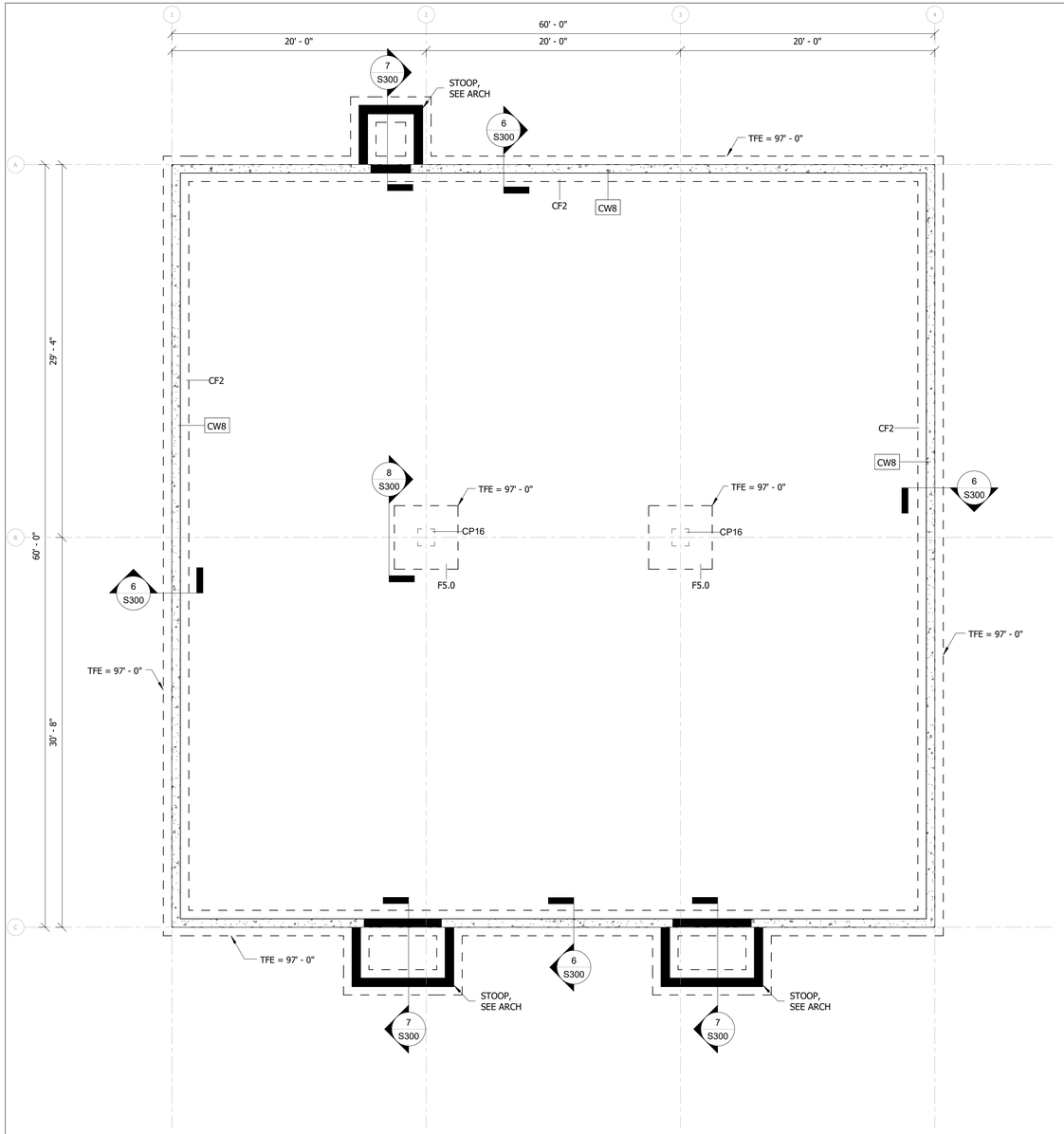
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GENERAL NOTES

DRAWING NO.

S100

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1 FOUNDATION PLAN
3/16" = 1'-0"

FOUNDATION PLAN NOTES:
UNLESS NOTED OTHERWISE:

1. SEE SHEET S000 FOR ABBREVIATIONS AND SYMBOLS.
2. SEE SHEET S100 FOR GENERAL NOTES.
3. SEE PLAN FOR TOP OF FOOTING ELEVATIONS.
4. TOP OF FOOTING ELEVATIONS:
   A. EXTERIOR FOOTINGS: 97'-0"
5. 'F#' ON PLAN DENOTES SPREAD FOOTING TYPE. SEE SCHEDULE ON SHEET S200 FOR ADDITIONAL INFORMATION. SPREAD FOOTINGS SHALL BE CENTERED ON COLUMNS AND WALL INTERSECTIONS UNLESS NOTED OTHERWISE ON PLAN.
6. 'CF#' ON PLAN DENOTES CONTINUOUS STRIP FOOTING. SEE SCHEDULE ON SHEET S200 FOR ADDITIONAL INFORMATION. STRIP FOOTINGS SHALL BE CENTERED ON FOUNDATION WALLS UNLESS NOTED OTHERWISE ON PLAN.
7. 'CP#' ON PLAN DENOTES CONCRETE PIER. SEE SCHEDULE ON SHEET S200 FOR ADDITIONAL INFORMATION.
8. 'CW#' ON PLAN DENOTES CAST-IN-PLACE (CIP) WALL TYPE. SEE SCHEDULE ON SHEET S200 FOR ADDITIONAL INFORMATION.
9. S---------S ON PLAN DENOTES FOOTING STEP. SEE 3/S300 FOR ADDITIONAL INFORMATION.
10. FOOTINGS FOR WALLS NOT NOTED SHALL BE 12" THICK WITH A MINIMUM PROJECTION OF 4" EACH SIDE. REINFORCE WITH (2) #5 CONTINUOUS. ADD (1) #5 CONT FOR EACH 6" WIDTH OVER 24".
11. SEE ARCHITECTURAL PLANS FOR ALL OPENING LOCATIONS, DIMENSIONS, STOOPS, AND INTERIOR PARTITIONS.
12. SEE ARCHITECTURAL DRAWINGS FOR FIRE RATINGS OF STRUCTURAL AND NON-STRUCTURAL ASSEMBLIES.
13. SEE DETAIL 3/S300 FOR TYPICAL CONCRETE COVER OVER UTILITY PIPE. COORDINATE PIPE LOCATIONS AND ELEVATIONS WITH CIVIL AND MECHANICAL DRAWINGS.
14. SEE DETAIL 4/S300 FOR OPENINGS IN CIP CONCRETE WALLS WITH A MAXIMUM CLEAR SPAN OF 4'-0". COORDINATE OPENING LOCATIONS AND ELEVATIONS WITH ARCHITECTURAL, CIVIL, MECHANICAL AND ELECTRICAL DRAWINGS.
15. ▬▬ ON PLAN DENOTES TOP OF FOUNDATION WALL RECESS. COORDINATE SIZE AND LOCATION WITH ARCHITECTURAL DRAWINGS.
16. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS.

LAP SPLICE SCHEDULE (UNCOATED GRADE 60 BARS IN CONCRETE)

CLASS 'A' TENSION LAP SPLICE

BAR SIZE	CONCRETE COMPRESSIVE STRENGTH, f'_c (PSI)							
	3,000	3,500	4,000	5,000	6,000	7,000	8,000	10,000
#3	1'-5"	1'-4"	1'-3"	1'-1"	1'-0"	0'-11"	0'-11"	0'-9"
#4	1'-10"	1'-9"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"	1'-0"
#5	2'-4"	2'-2"	2'-0"	1'-10"	1'-8"	1'-7"	1'-5"	1'-3"
#6	2'-9"	2'-7"	2'-5"	2'-2"	2'-0"	1'-10"	1'-9"	1'-6"
#7	4'-1"	3'-9"	3'-6"	3'-2"	2'-11"	2'-8"	2'-6"	2'-3"
#8	4'-7"	4'-3"	4'-0"	3'-7"	3'-3"	3'-0"	2'-10"	2'-6"
#9	5'-2"	4'-10"	4'-6"	4'-1"	3'-8"	3'-5"	3'-3"	2'-10"
#10	5'-9"	5'-4"	5'-0"	4'-6"	4'-1"	3'-9"	3'-7"	3'-2"
#11	6'-4"	5'-11"	5'-6"	5'-0"	4'-6"	4'-2"	3'-11"	3'-6"

CLASS 'B' TENSION LAP SPLICE

BAR SIZE	CONCRETE COMPRESSIVE STRENGTH, f'_c (PSI)							
	3,000	3,500	4,000	5,000	6,000	7,000	8,000	10,000
#3	1'-10"	1'-8"	1'-7"	1'-5"	1'-4"	1'-3"	1'-2"	1'-0"
#4	2'-5"	2'-3"	2'-1"	1'-11"	1'-9"	1'-7"	1'-6"	1'-4"
#5	3'-0"	2'-10"	2'-7"	2'-4"	2'-2"	2'-0"	1'-10"	1'-8"
#6	3'-7"	3'-4"	3'-1"	2'-10"	2'-7"	2'-5"	2'-3"	2'-0"
#7	5'-3"	4'-10"	4'-7"	4'-1"	3'-9"	3'-5"	3'-3"	2'-11"
#8	6'-0"	5'-7"	5'-3"	4'-8"	4'-3"	3'-11"	3'-9"	3'-3"
#9	6'-9"	6'-3"	5'-11"	5'-3"	4'-10"	4'-5"	4'-2"	3'-8"
#10	7'-6"	6'-11"	6'-6"	5'-10"	5'-4"	4'-11"	4'-8"	4'-1"
#11	8'-3"	7'-8"	7'-2"	6'-5"	5'-10"	5'-5"	5'-1"	4'-6"

CIP CONCRETE LAP SPLICE SCHEDULE NOTES:

1. THESE TABLES ARE BASED ON THE FOLLOWING ASSUMPTIONS:
   A. UNCOATED GRADE 60 REBAR IN GROUTED CONCRETE
   B. CLEAR SPACING OF BARS ≥ d_b
   C. CLEAR COVER ≥ d_b
   D. STIRRUPS OR TIES PROVIDED THROUGHOUT DEVELOPMENT LENGTH MINIMUM ≥ OR
      a. CLEAR SPACE OF BARS ≥ 2*d_b
      b. CLEAR COVER ≥ d_b
2. FOR OTHER CASES, MULTIPLY LENGTHS SHOWN BY 1.5
3. TOP BAR SPLICES:
   A. TOP BARS ARE SUCH THAT 12" OR MORE OF FRESH CONCRETE IS CAST BELOW THE SPLICE OR DEVELOPMENT LENGTH
   B. MULTIPLY LENGTHS SHOWN BY 1.3
4. FOR LIGHTWEIGHT CONCRETE, MULTIPLY LENGTHS SHOWN BY 1.3
5. FOR HIGHER GRADE STEEL, MULTIPLY LENGTHS SHOWN BY A RATIO OF HIGHER F_Y (KSI) OVER 1,160 (KSI). ALL OTHER FACTORS LISTED STILL APPLY.
6. FOR COMPRESSION LAP SPLICE, USE GREATER OF:
   A. (0.0009*F_Y - 24)*d_b
   B. 0.0005*F_Y*d_b
7. ABBREVIATED TERMS:
   A. d_b = BAR DIAMETER
   B. F_Y = YIELD STRENGTH OF REBAR
8. ALL REBAR LAP SPLICES SHALL BE 'CLASS B' UNLESS NOTED OTHERWISE ON PLAN.

CIP STRIP FOOTING SCHEDULE

MARK	FOOTING SIZE	CONTINUOUS REINFORCEMENT	TRANSVERSE REINFORCEMENT	COMMENTS
CF2	2' - 0" x 12"	(2) - #5	--	

CONTINUOUS FOOTING SCHEDULE NOTES UNO ON PLAN:

1. SEE GENERAL NOTES ON SHEET S100FOR ADDITIONAL REQUIREMENTS.
2. CONTINUOUS FOOTINGS SHALL BE CENTERED UNDER WALLS.
3. SEE SCHEDULE ON SHEET S200 FOR REBAR LAP SPLICES FOR COATED AND UNCOATED REBAR.
4. ALL REINFORCING IN BOTTOM OF FOOTING.

CIP PAD FOOTING SCHEDULE

MARK	FOOTING SIZE	BOTTOM REINFORCEMENT	TOP REINFORCEMENT	COMMENTS
F5.0	5'-0" x 5'-0" x 1'-0"	(5) - #5 EACH WAY	N/A	

SPREAD FOOTING SCHEDULE NOTES UNO ON PLAN:

1. SEE GENERAL NOTES ON SHEET S100FOR ADDITIONAL REQUIREMENTS.
2. SPREAD FOOTINGS SHALL BE CENTERED UNDER COLUMNS.
3. SEE SCHEDULE ON SHEET S200 FOR REBAR LAP SPLICES FOR COATED AND UNCOATED REBAR.

CIP CONCRETE PIER SCHEDULE

MARK	SIZE	VERTICAL REINFORCEMENT	TIE REINFORCEMENT	TOP OF PIER	TYPE	COMMENTS
CP16	16" x 16"	(4) #6 BARS	#3 BARS AT 12" OC	99'-0"	'A'	

CIP CONCRETE PIER SCHEDULE NOTES UNO ON PLAN:

1. SEE GENERAL NOTES ON SHEET S100FOR ADDITIONAL REQUIREMENTS.
2. PIERS SHALL BE LOCATED AT COLUMN CENTERLINE ON PLAN AND DETAILS.
3. PROVIDE CP_ PIER WHERE CIP CONCRETE PIER MARK ARE NOT INDICATED ON PLAN. CONFIRM WITH SEOR PRIOR TO PLACEMENT.
4. POUR CIP CONCRETE PIERS INTEGRALLY WITH CIP CONCRETE WALLS.
5. PROVIDE STANDARD 90° HOOK DOWELS BETWEEN FOUNDATIONS AND CIP COLUMNS EQUAL TO THE SIZE AND SPACING OF THE VERTICAL REINFORCING. PLACE HOOKED END OF DOWEL 3" FROM BOTTOM OF FOOTING AND PROJECT CLASS 'A' LAP ABOVE TOP OF FOOTING.
6. EXTEND VERTICAL BARS TO 2" CLEAR TOP OF PIER.
7. SEE SCHEDULES ON SHEET S200 FOR REBAR LAP SPLICES FOR COATED AND UNCOATED REBAR. PROVIDE CLASS 'B' TENSION LAP SPLICE.
8. ALL PIERS SHALL HAVE (3) ADDTIONAL TIES IN TOP 5" PORTION OF PIERS PER ACI 10.7.6.1.6.
9. PROVIDE CROSSTIES AS SHOWN WITH 90° HOOK ONE END AND 135° HOOK OTHER END. SIZE TO MATCH TIES. ALTERNATE TIE CONFIGURATIONS MEETING ACI 318 SECTION 25.7.2.3 ARE ACCEPTABLE.
10. PROVIDE FIRST TIE MAXIMUM OF 1/2 TIE SPACING ABOVE TOP OF FOOTING.
11. REFER TO DRAWINGS FOR TOP OF PIER ELEVATIONS.



TYPE 'A'

CIP CONCRETE WALL SCHEDULE

MARK	WALL THICKNESS	VERTICAL REINFORCEMENT	HORIZONTAL REINFORCEMENT	REINFORCEMENT LOCATION	COMMENTS
CW8	8"	#4 BARS @ 18" OC	#4 BARS @ 12" OC	CENTER	

CIP CONCRETE WALL SCHEDULE NOTES UNO ON PLAN:

1. SEE GENERAL NOTES ON SHEET S100FOR ADDITIONAL REQUIREMENTS.
2. PROVIDE MARK CW_ WHERE CIP CONCRETE WALL MARK IS NOT SHOWN. CONFIRM WITH ENGINEER PRIOR TO PLACEMENT.
3. EXTEND VERTICAL REINFORCING FROM FOOTINGS TO 2" CLEAR TOP OF WALL OR TO BEAM BEARING.
4. WALL REINFORCING IS CONTINUOUS THROUGH COLUMNS, PIERS, AND LINTELS.
5. SEE DETAIL 2/S300 FOR CONTROL JOINT REQUIREMENTS IN CIP WALLS.
6. SEE FOUNDATION AND WALL DETAILS IN 9/S300 FOR DETAILING AT JOINTS, CORNERS, AND INTERSECTIONS.
7. VERTICAL REINFORCING IS PRIMARY. WHEN INDICATED AS INSIDE FACE ABOVE, PLACE VERTICAL REINFORCING CLOSEST TO INSIDE (INTERIOR FACE OF WALL AND HORIZONTAL REINFORCING BEHIND THAT). WHERE CENTERED IS INDICATED ABOVE, CENTER THE VERTICAL REINFORCING AND PLACE HORIZONTAL REINFORCING ON EITHER SIDE OF THE VERTICAL BARS.
8. SEE SCHEDULES ON SHEET S200 FOR REBAR LAP SPLICES FOR COATED AND UNCOATED REBAR.
9. REFER TO ARCHITECTURAL DRAWINGS FOR VENEER SHELF ELEVATIONS AT FOUNDATION WALL.
10. PROVIDE STANDARD 90-DEGREE HOOK DOWELS BETWEEN FOUNDATIONS AND WALLS EQUAL TO THE SIZE AND SPACING OF THE VERTICAL REINFORCING. PLACE HOOKED END OF DOWEL 3" FROM BOTTOM OF FOOTING AND PROJECT CLASS 'A' LAP ABOVE TOP OF FOOTING.
11. SLEEVES EMBEDDED IN WALLS SHALL BE LOCATED CLEAR BETWEEN REINFORCING BARS AND SHALL MAINTAIN CLEAR SPACING EQUAL TO THE DIAMETER OF THE LARGEST SLEEVE IN ANY DIRECTION. SLEEVE GROUPS THAT DO NOT COMPLY WITH THE ABOVE REQUIREMENTS SHALL BE CONSIDERED AS AN OPENING AND REINFORCED PER NOTE #12 BELOW.
12. PROVIDE EXTRA REINFORCING ON ALL SIDES OF ALL MISCELLANEOUS WALL OPENINGS EQUAL TO ONE HALF THE INTERRUPTED REINFORCING BARS EACH SIDE BUT NOT LESS THAN (2) #5 FOR EACH LAYER OF REINFORCEMENT. EXTEND BARS CLASS 'B' LAP LENGTH BUT NOT LESS THAN 2 FEET BEYOND EDGE OF OPENINGS. PROVIDE (2) #4 x 48" DIAGONAL BARS AT EACH CORNER FOR EACH LAYER OF REINFORCEMENT.

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Corporate Office:
120 17th Ave W Alexandria, MN 56308
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PRINTED NAME: JOE A NICHOLSON

DATE: 9/18/2026 LICENSE #: 44104

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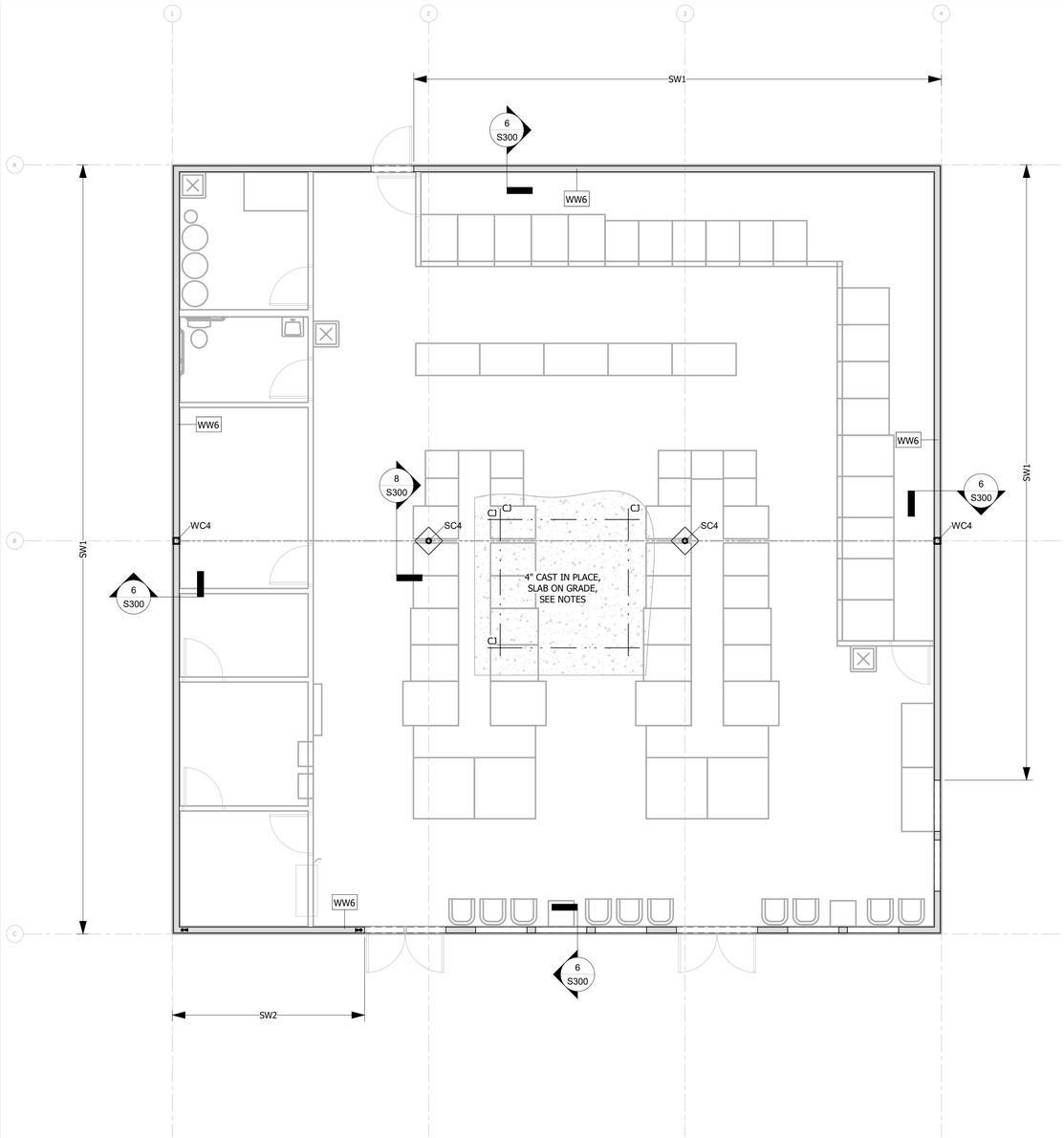
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FOUNDATION PLAN

DRAWING NO.

S200

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1 SLAB ON GRADE PLAN
3/16" = 1'-0"

SLAB ON GRADE PLAN NOTES:
UNLESS NOTED OTHERWISE:
1. TOP OF INTERIOR SLAB ELEVATION = 100' - 0" . SEE ARCHITECTURAL DRAWINGS FOR SLAB SLOPES, STEPS, RAMPS, AND ELEVATIONS.
2. INTERIOR SLAB-ON-GRADE SHALL BE 4" THICK REINFORCED W/SYNTHETIC MICRO FIBER REINFORCING (MIN 1.5 LB/CY) ON A MINIMUM 10-MIL VAPOR BARRIER COMPLYING W/ ASTM E1745, CLASS A (MAXIMUM 0.04 PERM).
   SEE GEOTECHNICAL REPORT FOR ADDTIONAL INFORMATION.
3. SLAB CONTROL JOINTS:
   A. MAXIUMUM SPACING = 10' - 0"
   B. MAXIMUM PANEL ASPECT RATIO = 1.5 (LONG SIDE / SHORT SIDE)
   C. VERIFY JOINT LOCATIONS WITH ARCH
   D. SEE DETAIL 1/S300 FOR ADDTIONAL INFORMATION.
4. SEE ARCHITECTURAL PLANS FOR ALL OPENING LOCATIONS, DIMENSIONS, STOOPS, AND INTERIOR PARTITIONS.
5. ⟷ ON PLAN DENOTES SHEAR WALL HOLD DOWNS. SEE SCHEDULE ON SHEET S302 FOR ADDITIONAL INFORMATION.
6. ⟵SWx⟶ ON PLAN DENOTES SHEAR WALL LOCATIONS, SEE SCHEDULE ON SHEET S302 FOR ADDITIONAL INFORMATION.

WOOD WALL SCHEDULE

MARK	STUD SIZE	STUD SPACING	BOTTOM PLATE	TOP PLATE	COMMENTS
WW6	2 x 6		(1) 2 x 6 PT	(2) 2 x 6	

WOOD WALL SCHEDULE NOTES UNO ON PLAN:

1. SEE ARCH DRAWINGS FOR INTERIOR AND EXTERIOR SHEATHING UNLESS SPECIFIED IN SHEAR WALL SCHEDULE ON SHEET S302 .
2. SEE WOOD FRAMING SPECIES AND GRADE SCHEDULE ON SHEET S100FOR ADDTIONAL INFORMATION.
3. SEE DETAIL 2/S301 FOR TYPICAL WALL INTERSECTION DETAILS.
4. SEE DETAIL 3/S301 FOR BUILT-UP COLUMN NAILING.
5. SEE DETAIL 1/S301 FOR TYPICAL SOLE PLATE ANCHORAGE ON PLANS, SCHEDULES AND DETAILS.
6. SEE WOOD SHEAR WALL SCHEDULE FOR WALLS THAT REQUIRE BLOCKING.
7. WHERE WALL STUDS ARE SPACED AT 24" OC, FLOOR AND ROOF TRUSSES MUST BEAR DIRECTLY ABOVE STUDS BELOW TOP PLATE. ADD ADDITIONAL STUDS IF REQUIREMENT IS NOT MET.
8. SEE ARCH DRAWINGS FOR ALL NON-BEARING AND PARTITION WALL FRAMING NOT SHOWN ON STRUCTURAL DRAWINGS.
9. SEE DETAIL 6/S301 FOR TYPICAL WOOD WALL FRAMING.

WOOD COLUMN SCHEDULE

MARK	SIZE	POST BASE	POST CAP	COMMENTS
WC4	(4) 2 x 6	SOLE PLATE	ECCQ	

WOOD COLUMN SCHEDULE NOTES UNO ON PLAN:

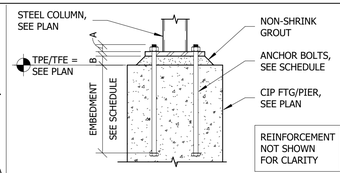
1. SEE DETAILS 8/S301 & 9/S301 FOR BEAM TO POST CONNECTIONS.
2. SEE GENERAL NOTES ON SHEET S100 FOR ADDITIONAL INFORMATION.
3. SEE 3/S301 FOR BUILT-UP COLUMN NAILING SCHEDULE.

STEEL COLUMN SCHEDULE

MARK	COLUMN	BASE PLATE	BASE PLATE TYPE	ANCHOR RODS	EMBEDMENT	COMMENTS
SC4	4"Ø STD PIPE	3/4" x 10" x 0'-10"	'A'	(4) 3/4"Ø	9"	

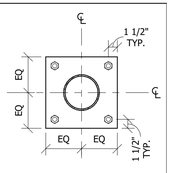
STEEL COLUMN SCHEDULE NOTES:

1. SEE GENERAL NOTES ON SHEET S100FOR ADDITIONAL MATERIAL REQUIREMENTS.
2. ANCHOR ROD EMBEDMENT DOES NOT INCLUDE PROJECTION.
3. PROVIDE HEAVY HEX NUTS INTO AT BASE OF ANCHORS RODS IN CONCRETE, TACK WELD NUT TO ROD.
4. UPSET THREADS AFTER COLUMN IS LEVELED AND IN FINAL POSITION.
5. PROVIDE FLAT WASHER BETWEEN ANCHOR ROD AND BASE PLATE.
6. PROVIDE 5/16" OVERSIZED HOLE IN BASE PLATE FOR ANCHOR ROD PLACEMENT.
7. PROVIDE PROTECTIVE ASPHALTIC COATING OR EQUAL AROUND STRUCTURAL STEEL BELOW GRADE.
8. GROUTING UNDER COLUMN BASE PLATES SHALL BE ACCOMPLISHED WITH NONMETALLIC, SHRINK RESISTING GROUT, ASTM C1103 WITH A MINIMUM COMPRESSIVE STRENGTH OF 7,000 PSI IN ACCORDANCE WITH ACI 351.1.



ANCHOR BOLT EMBEDMENT

A. ROD	A	B
1/2"Ø	1 1/2"	1 1/2"
5/8"Ø	2"	1 1/2"
3/4"Ø	2 1/4"	1 1/2"
1"Ø	3"	2"
1 1/4"Ø	3 3/4"	2 1/2"
1 1/2"Ø	4 1/2"	3"



TYPE 'A'

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Corporate Office:
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888-216-1916

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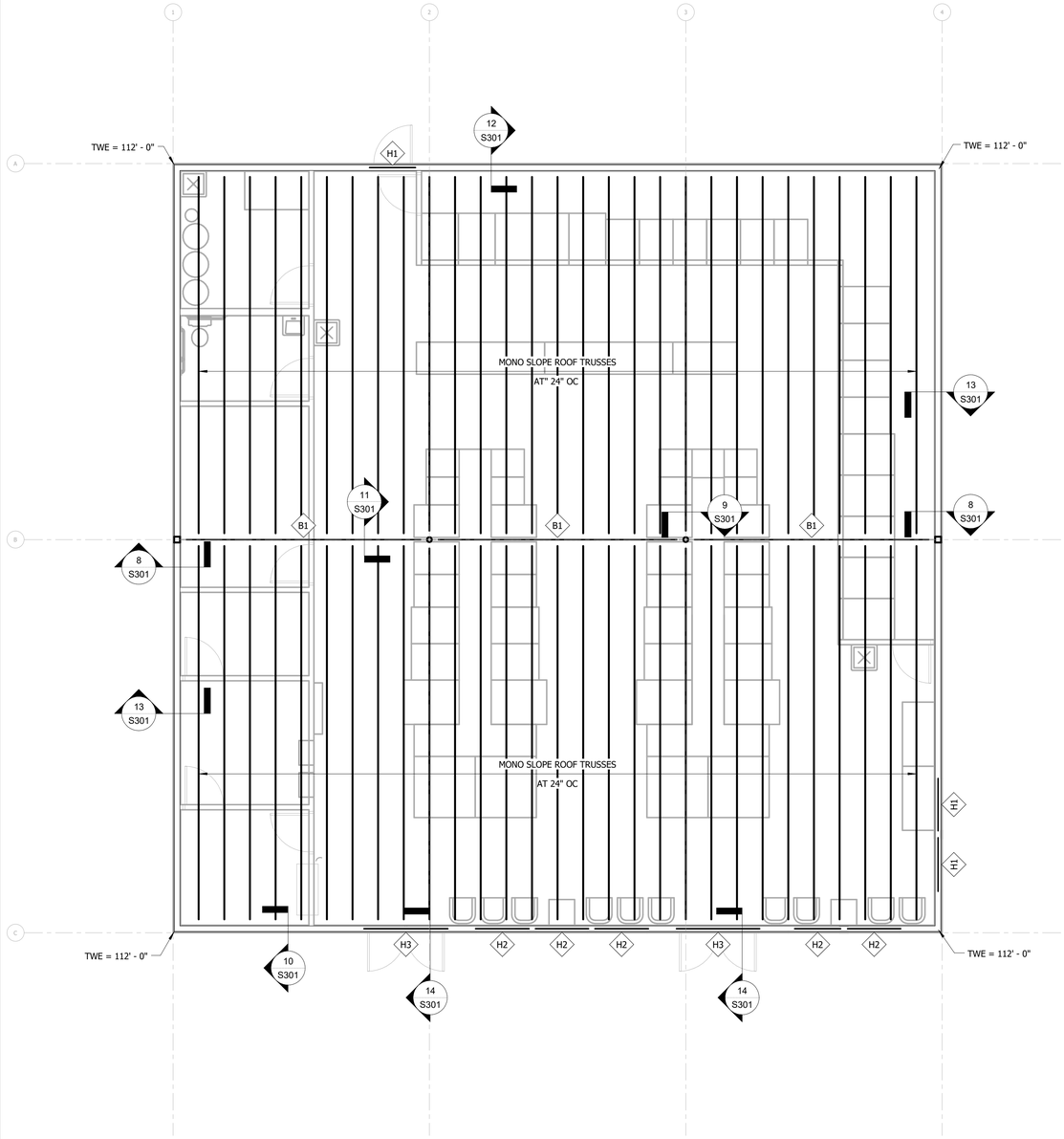
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SLAB ON GRADE PLAN

DRAWING NO.

S205

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1 ROOF FRAMING PLAN
3/16" = 1'-0"

ROOF PLAN NOTES:

1. SEE ARCHITECTURAL DRAWINGS FOR ROOF PITCHES AND ELEVATIONS.
2. REFER TO SHEET S301 FOR TYPICAL FRAMING DETAILS.
3. THE CONTRACTOR SHALL COORDINATE BETWEEN THE TRUSS MANUFACTURER AND MECHANICAL DRAWINGS/CONTRACTOR TO ENSURE THAT PROPRIETARY ELEMENTS ARE DESIGNED TO RESIST SUSPENDED LOADS FROM THEIR EQUIPMENT. THE WEIGHT OF SUSPENDED MECHANICAL LOADS HAS NOT BEEN ACCOUNTED FOR IN THE JOIST DESIGN INDICATED ON PLANS.
4. SEE ARCHITECTURAL DRAWINGS FOR ROOF TRUSS BEARING ELEVATION, DIMENSION, AND TRUSS PROFILE.
5. SEE ARCHITECTURAL DRAWINGS FOR ALL OPENING DIMENSIONS AND LOCATIONS.
6. TRUSS BRACING BY TRUSS SUPPLIER.
7. WOOD ROOF TRUSS NOTES:
   a. ROOF TRUSSES ARE PITCHED TOP CHORD OPEN WEB TRUSSES AT 24" OC MAX. SPACING MAY BE REDUCED BY TRUSS SUPPLIER.
   b. SEE ARCH FOR PITCHED BOTTOM CHORD LOCATIONS.
   c. SEE SHEET S100 FOR LOADS.
   d. ROOF TRUSSES SHALL BE DESIGNED FOR A MAXIMUM LL DEFLECTION = SPAN/360 AND A MAXIMUM TL DEFLECTION = SPAN/240.
8. TRUSS SUPPLIER SHALL PROVIDE HURRICANE TIES TO TOP PLATE AT ALL ROOF FRAMING MEMBERS.
9. ROOF SHEATHING:
   A. AT ROOF TRUSS FRAMING: 19/32" PLYWOOD SHEATHING. SEE DETAIL 5/S302.
10. REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS.

WOOD BEAM SCHEDULE

MARK	BEAM SIZE	CONNECTION	COMMENTS
B1	(3) 1 3/4" x 22" LVL	CCQ/ECCQ	

WOOD BEAM SCHEDULE NOTES UNO ON PLAN:

1. 'B1' IN SCHEDULE DENOTES BEAMS.
2. SEE SCHEDULE ON SHEET S301 FOR BUILT-UP BEAM FASTENING SCHEDULE.
3. SEE 3/S301 FOR BUILT-UP COLUMN NAILING SCHEDULE.
4. SEE WOOD FRAMING SPECIES AND GRADE SCHEDULE ON SHEET S100 FOR ADDITIONAL INFORMATION.
5. SIZES AND QUANTITIES PROVIDED ARE MINIMUM ALLOWED. STRONGER MEMBERS MAY BE SUBSTITUTED PROVIDED THE PERFORMANCE OF THE ALTERNATE MEMBER EXCEEDS THE MEMBER SHOWN HERE. ALTERNATE DESIGNS MUST BE SUBMITTED TO AND APPOROVED IN WRITING BY THE ENGINEER PRIOR TO CONSTRUCTION.
6. DIMENSIONAL LUMBER IS NOT ALLOWED BETWEEN SCL BEAMS AND SCL POSTS CONNECTIONS.

WOOD HEADER SCHEDULE

MARK	HEADER SIZE	JACK STUDS	KING STUDS	COMMENTS
H1	(2) 2 x 8	(1) 2x	(1) 2x	
H2	(2) 2 x 8	(1) 2x	(2) 2x	
H3	(2) 1 3/4" x 9 1/4" LVL	(2) 2x	(2) 2x	

WOOD HEADER SCHEDULE NOTES UNO ON PLAN:

1. 'H1' IN SCHEDULE DENOTES DROPPED HEADERS INSTALLED OVER WINDOWS, DOORS, AND MECH OPENINGS.
2. SEE SCHEDULE ON SHEET S100 FOR BUILT-UP HEADER FASTENING SCHEDULE.
3. SEE 3/S301 FOR BUILT-UP COLUMN NAILING SCHEDULE.
4. 2x JACK / POST & KING STUDS SHALL MATCH WALL THICKNESS.
5. SEE WOOD FRAMING SPECIES AND GRADE SCHEDULE ON SHEET S100 FOR ADDITIONAL INFORMATION.
6. SIZES AND QUANTITIES PROVIDED ARE MINIMUM ALLOWED. STRONGER MEMBERS MAY BE SUBSTITUTED PROVIDED THE PERFORMANCE OF THE ALTERNATE MEMBER EXCEEDS THE MEMBER SHOWN HERE. ALTERNATE DESIGNS MUST BE SUBMITTED TO AND APPOROVED IN WRITING BY THE ENGINEER PRIOR TO CONSTRUCTION.
7. IF PLANS INDICATED THE PRESENCE OF A SCL COLUMN, COLUMN TAKES PRECEDENCE OF OVER JACK STUDS.
8. DIMENSIONAL LUMBER IS NOT ALLOWED BETWEEN SCL HEADERS AND SCL POSTS CONNECTIONS.
9. SEE DETAIL 6/S301 FOR HEADER IN TYPICAL WOOD FRAMED WALL.

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120 17th Ave W Alexandria, MN 56308
888-216-1916

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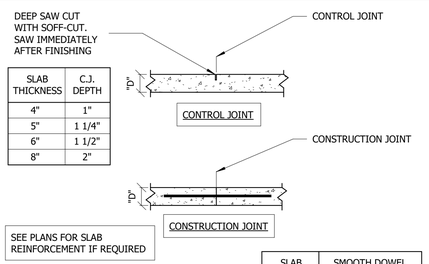
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ROOF FRAMING PLAN

DRAWING NO.

S210

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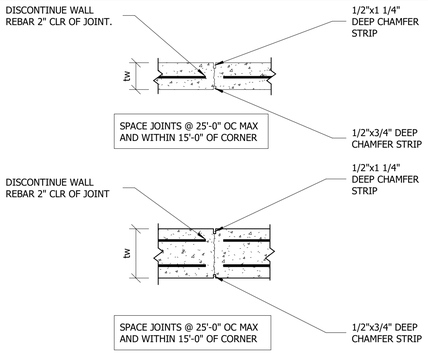


SLAB THICKNESS	C.J. DEPTH
4"	1"
5"	1 1/4"
6"	1 1/2"
8"	2"

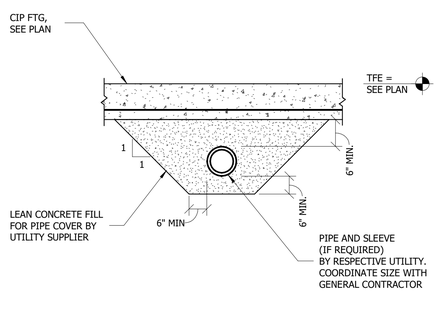
SMOOTH DOWELS NOTES:
1. CENTER DOWELS IN SLAB THICKNESS
2. SEE SMOOTH DOWEL TABLE FOR SIZE, LENGTH AND SPACING REQUIREMENTS
3. ALL DOWELS SHALL BE SAW CUT NOT SHEARED AND CENTERED IN SLAB THICKNESS

SLAB THICKNESS	SMOOTH DOWEL SIZE	LENGTH	SPC'G
4"	1/2"Ø	2'-0"	16" OC
5"	3/4"Ø	2'-0"	12" OC
6"	3/4"Ø	2'-0"	12" OC
8"	1"Ø	2'-0"	12" OC

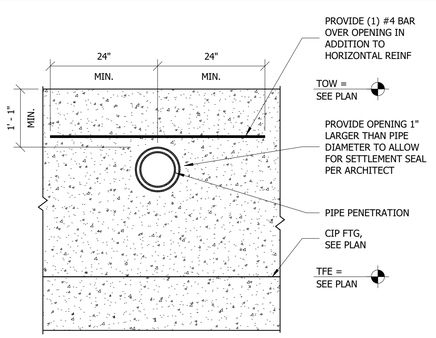
1 CONTROL JOINT/CONSTRUCTION JOINT
NO SCALE



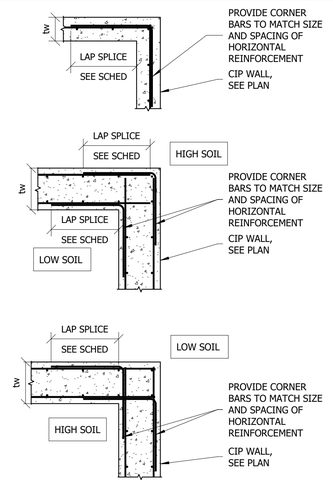
2 CIP WALL CONTROL JOINT
NO SCALE



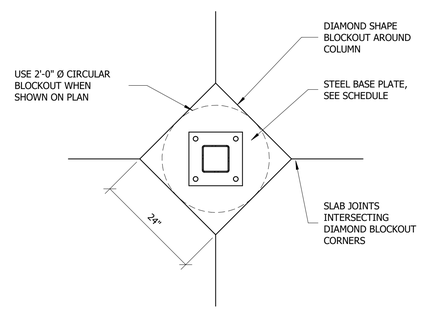
3 CONCRETE COVER FOR UTILITY PIPE
NO SCALE



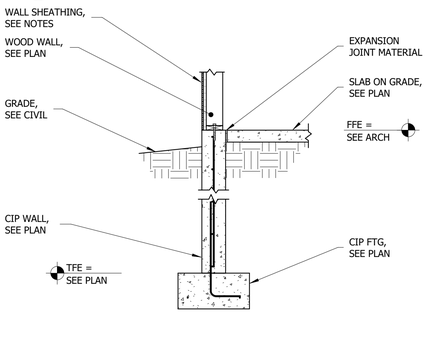
4 SECTION @ PIPE PENETRATION
NO SCALE



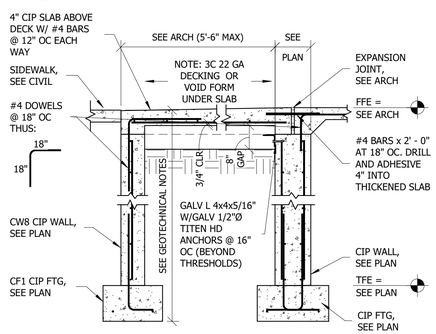
9 CIP WALL CORNER REINFORCEMENT
NO SCALE



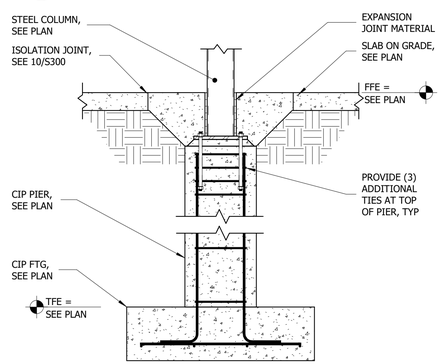
5 TYPICAL STEPPED FTG
NO SCALE



6 CIP FOUNDATION WOOD WALL
NO SCALE



7 CIP STOOP DETAIL
NO SCALE



8 ELEVATION VIEW OF PIER
NO SCALE

10 COLUMN ISOLATION JOINT
NO SCALE

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DATE: 9/18/2026 LICENSE #: 44104

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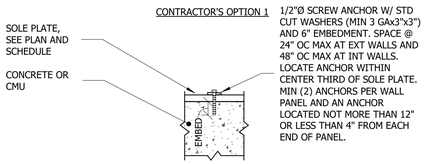
NO.	DATE	DESCRIPTION

FOUNDATION DETAILS - CIP

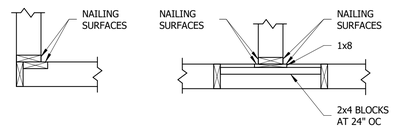
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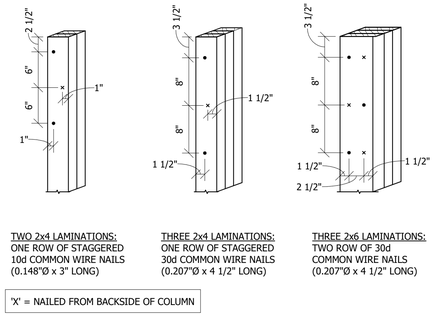
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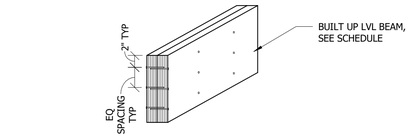
1 TYPICAL SOLE PLATE ANCHORAGE
NO SCALE



2 TYPICAL WALL INTERSECTIONS
NO SCALE



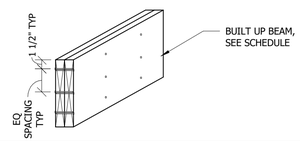
3 BUILT-UP COLUMN NAILING
NO SCALE



PLY WIDTH	NUMBER OF PLIES	FASTENER				
		TYPE[1]	MIN LENGTH	# OF ROWS	SPACING	LOCATION
1 3/4"	2	10d[2]	3"	4	12" OC	BOTH SIDES
		16d[2]	3 1/2"	3	12" OC	BOTH SIDES
		SCREWS[3]	3 3/8"	2	16" OC	BOTH SIDES
	3	10d[2]	3"	4	6" OC	BOTH SIDES
		16d[2]	3 1/2"	5	12" OC	BOTH SIDES
		SCREWS[3]	5"	3	16" OC	BOTH SIDES
	4	SCREWS[3]	6 3/4"	3	16" OC	BOTH SIDES

NOTES:
1. WHERE FRAMING IS HANGERED TO BEAM, USE SCREW FASTENER OPTION ONLY. CONTRACTOR'S OPTION ON FASTENER AT ALL OTHER CONDITIONS.
2. NAIL DIAMETER IS TO MATCH COMMON NAIL OF SIZE NOTED IN TABLE.
3. SCREWS ARE TO BE SIMPSON SDW SCREWS OR APPROVED EQUAL.

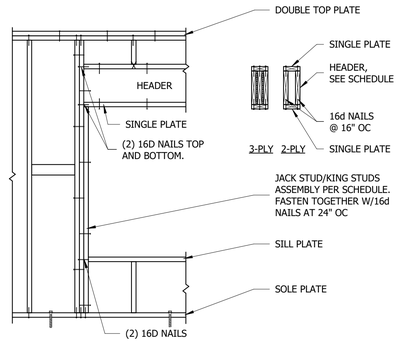
4 BUILT UP LVL BEAM FASTENING SCHEDULE
NO SCALE



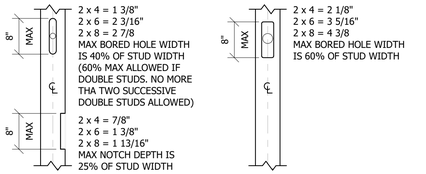
PLY WIDTH	NUMBER OF PLIES	FASTENER				
		TYPE[1]	MIN LENGTH	# OF ROWS	SPACING	LOCATION
1 1/2"	2	10d[2]	3"	4	12" OC	BOTH SIDES
		16d[2]	3"	3	8" OC	BOTH SIDES
		SCREWS[3]	3"	2	16" OC	BOTH SIDES
	3	10d[2]	3"	4	6" OC	BOTH SIDES
		16d[2]	3 1/2"	5	8" OC	BOTH SIDES
		SCREWS[3]	4 3/8"	3	16" OC	BOTH SIDES
	4	SCREWS[3]	6"	2	16" OC	BOTH SIDES

NOTES:
1. CONTRACTOR'S OPTION ON FASTENER.
2. NAIL DIAMETER IS TO MATCH COMMON NAIL OF SIZE NOTED IN TABLE.
3. SCREWS ARE TO BE SIMPSON SDW SCREWS OR APPROVED EQUAL.

5 BUILT UP WOOD BEAM FASTENING
NO SCALE

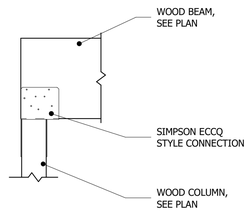


6 TYPICAL SINGLE STORY WOOD FRAMED WALL
NO SCALE

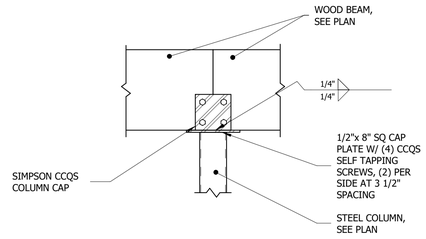


NOTES:
1. HOLES SHALL BE LOCATED IN THE CENTER OF WALL STUDS.
2. HOLES SHALL NOT BE LOCATED IN THE SAME STUD AS A CUT OR NOTCH.
3. CONTACT ARCHITECT OR STRUCTURAL ENGINEER PRIOR TO CUTTING OR NOTCHING IF HOLES GREATER THAN 20% STUD WIDTH OR NOTCHES GREATER THAN 10% STUD WIDTH ARE REQUIRED IN TWO OR MORE CONSECUTIVE STUDS.
4. STUD SIZES NOTED ON STRUCTURAL DRAWINGS ARE THE MINIMUM REQUIRED FOR STRUCTURE. COORDINATE WITH MEP TO IDENTIFY STRUCTURAL WALL LOCATIONS WHERE STUD PENETRATIONS EXCEED THE ALLOWABLE HOLE OR NOTCH SIZES SHOWN. INCREASE STUD SIZE TO MEET ALLOWABLE HOLE OR NOTCH REQUIREMENTS. COORDINATE REVISED STUD SIZES WITH ARCHITECT AND STRUCTURAL ENGINEER.
5. PROVIDE GAP ABOVE CONDUIT PIPE FOR DIFFERENTIAL MOVEMENT. SEE GENERAL STRUCTURAL NOTES FOR ANTICIPATED WOOD SHRINKAGE. COORDINATE WITH MEP FOR ANTICIPATE MOVEMENT OF CONDUIT/PIPE AND REQUIRED GAP BELOW.

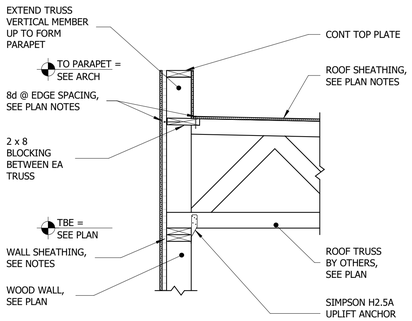
7 HOLES AND NOTCHES IN WOOD STUDS
NO SCALE



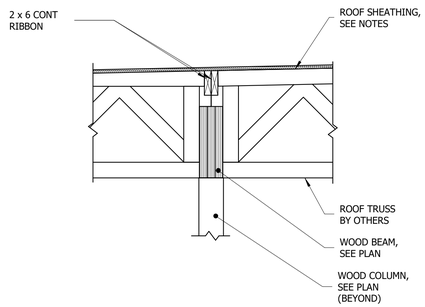
8 NEW BEAM BEARING END
NO SCALE



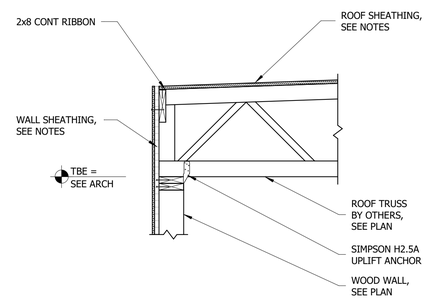
9 NEW BEAM BEARING MID
NO SCALE



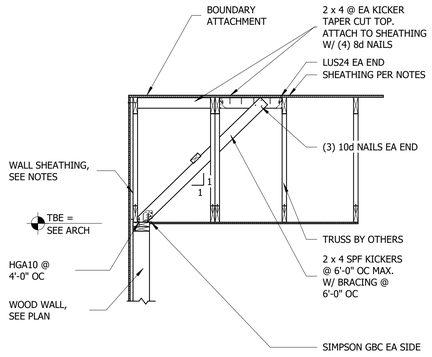
10 ROOF MONO TRUSS BEARING HIGH
NO SCALE



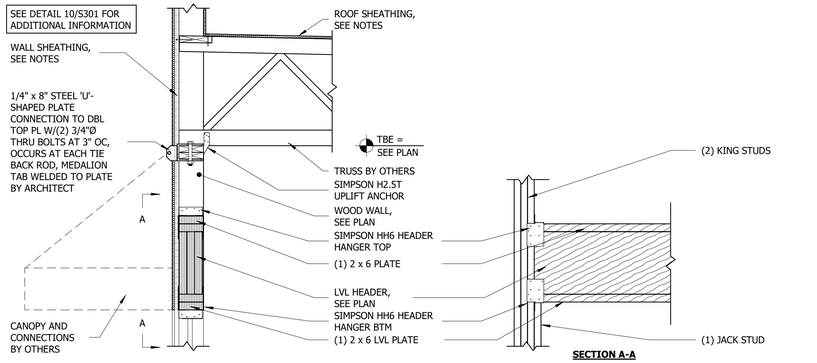
11 ROOF TRUSS BEARING MID AT BEAM
NO SCALE



12 ROOF MONO TRUSS BEARING EAVE
NO SCALE



13 ROOF TRUSS LAP SIDE
NO SCALE



14 CANOPY SUPPORT DETAIL
NO SCALE

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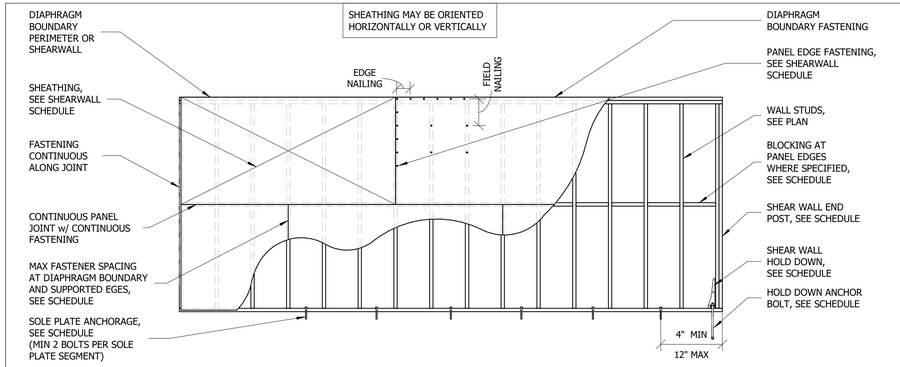
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FRAMING DETAILS - WOOD

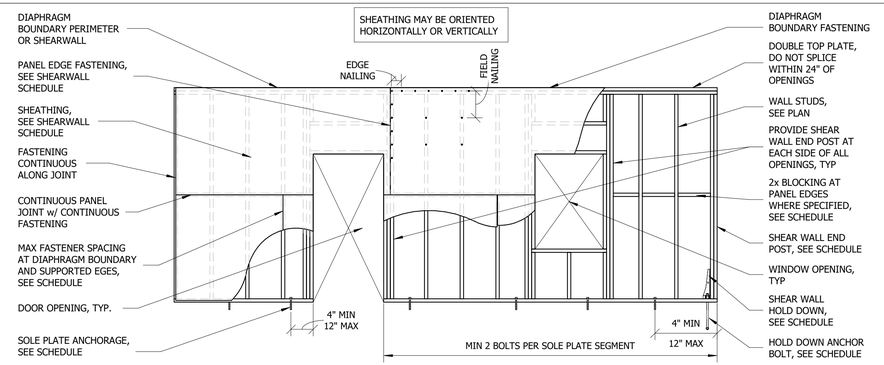
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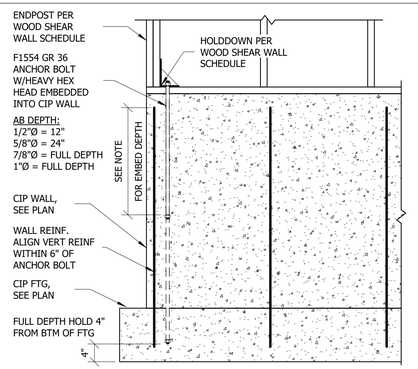
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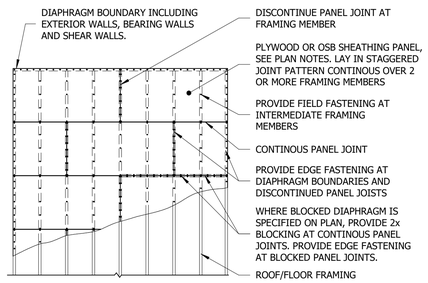
1 SEGMENTED SHEAR WALL
NO SCALE



2 PERFORATED SHEAR WALL
NO SCALE

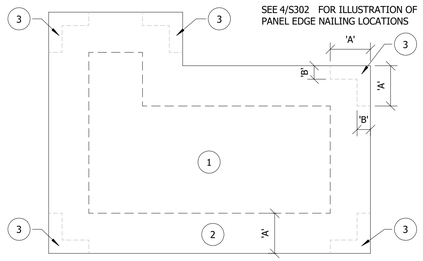


3 END OF WALL HOLDDOWN ANCHOR
NO SCALE



FLOOR/ROOF SHEATHING FASTENING SCHEDULE		
PANEL LOCATION	TYPICAL FLOOR	TYPICAL ROOF
EDGE	0.131"x2 1/2" NAILS @ 6"OC	SEE 5/S302
FIELD	0.131"x2 1/2" NAILS @ 12"OC	

4 FLOOR/ROOF SHEATHING FASTENING DIAGRAM
NO SCALE



ROOF SHEATHING FASTENING SCHEDULE						
NAILS	'A'	'B'	PANEL LOCATION	ROOF FASTENING ZONE		
				1	2	3
8d COMMON (0.131"Øx2 1/2")	8'-6"	3'-0"	PANEL EDGES	6"OC	6"OC	4"OC
			PANEL FIELD	12"OC	6"OC	6"OC

5 FLAT ROOF SHEATHING FASTENING
NO SCALE

WOOD SHEAR WALL SCHEDULE

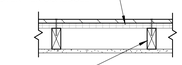
MARK	LEVEL	ANCHOR POSTS & ANCHORAGE					SHEATHING				COMMENTS
		END POST SIZE	HOLD DOWN OR STRAP	FASTENERS			TYPE	FASTENING		IS WALL BLOCKED?	
				ANCHOR BOLT	STRAP NAILING (END ZONE/TOTAL)	SOLE PLATE ANCHORAGE		EDGE	FIELD		
SW1	1	(2) 2x	--	--	--	1/2"Ø AT 48" OC	Z1	0.131"Ø @ 4" OC	0.131"Ø @ 12" OC	YES	
SW2	1	(2) 2x	HDUE3-SDS3	--	--	1/2"Ø AT 48" OC	Z1	0.131"Ø @ 3" OC	0.131"Ø @ 12" OC	YES	

WOOD SHEAR WALL SCHEDULE NOTES:

1. ZIP SHEATHING SHEAR WALL NOTES, SEE ARCH FOR WALL TYPES.
2. WALL SHEATHING FASTENING SHALL MEET THE FOLLOWING REQUIREMENTS:
   A. SINGLE LAYER R3 ZIP SHEATHING:
      a. 0.131"Ø SHANK NAILS, MIN 1 1/2" PENETRATION INTO WALL FRAMING.
3. HOLD DOWNS OR STRAPS (IF REQUIRED) ARE LOCATED AT THE ANCHOR POST LOCATIONS AT EACH END OF SHEAR WALL.
4. AT VENT OPENINGS WITHIN SHEAR WALLS LESS THAN 8" IN DIAMETER, PROVIDE SOLID BLOCKING AROUND ALL SIDES OF VENT. FASTEN SHEATHING TO BLOCKING USING EDGE NAILING PATTERN SHOWN IN SHEAR WALL SCHEDULE. FOR OPENINGS GREATER THAN 8" IN DIAMETER, NOTIFY ENGINEER OF CONDITION IN WRITING FOR FUTHER INSTRUCTION PRIOR TO PROCEEDING WITH WORK.
5. SHEAR WALL SHEATHING ASSEMBLY TYPE NOTES:
   A. NOT ALL CONDITIONS PRESENTED BELOW WILL BE USED. REFER TO SHEAR WALL SCHEDULE FOR SPECIFIC TYPES REQUIRED.
   B. SHEAR WALL SHEATHING AND FASTENING TO TAKE PRECEDENCE OVER ARCHITECTURAL REQUIREMENTS.
   C. CONDITIONS PRESENTED ARE NOT INTENDED TO CAPTURE ALL POSSIBLE VARIATIONS OF FINISH MATERIALS AND LAYERS OF EXCESS GYP. BOARD USED FOR FIRE RATING. LAYERS IN EXCESS OF THAT SHOWN IN SCHEDULE ARE NOT TO BE USED FOR SHEAR WALL STRENGTH AND CAN USE NOMINAL FASTENING.

SHEAR WALL ZIP SHEATHING ASSEMBLY

DESIGNATED SHEAR WALL SHEATHING:
(1) LAYER OF R3 ZIP SHEATHING ONE SIDE, FASTEN PER SCHEDULE



NOTES:
1. SEE ARCH FOR WALL ASSEMBLY AND ATTACH PER GENERAL NOTES

Z1 TYPE 'Z1'

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SHEAR WALL AND SHEATHING DETAILS

DRAWING NO.

S302

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